



**HYDROGEOLOGICAL REVIEW
MABERLY PINES SUBDIVISION
CONTRACT #2021-PD-002**

Submitted to:

The Corporation of Tay Valley Township
217 Harper Road
Perth, ON K7H 3C6

Submitted by:

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BluMetric Project Number: 220037

November 30, 2021

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Attention: Amanda Mabo, Clerk

clerk@tayvalleytwp.ca

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1. INTRODUCTION

BluMetric Environmental Inc. (BluMetric™) was retained by Tay Valley Township (TVT) to conduct a review of hydrogeological conditions at the Maberly Pines Subdivision located approximately three kilometres south of the Village of Maberly, Ontario, on the northeast side of Bolingbrook Road (see Figure 1).

The Maberly Pines Subdivision was investigated by Water and Earth Science Associates Limited (WESA) in 1979 (report titled Maberly Pines Subdivision, Terrain, Hydrogeological and Ecological Analysis). The development was subsequently approved, and all 56 lots were sold to individual owners. Since that time, five of the lots have been developed and two have been issued building permits. There are currently 49 vacant lots issued through By-Law NO. 2021-033 to Amend By-Law No. 2002-121, as amended Plan 21 Lakeside Living (Maberly Pines) (Geographic Township of South Sherbrooke).

Recent concerns regarding development lead TVT to consult with the Mississippi Valley Conservation Authority (MVCA) and the Rideau Valley Conservation Authority (RVCA) regarding approval based on current guidelines. It was noted that the WESA, 1979 report does not fully address the current guidelines regarding the assessment of water quality, quantity, and nitrate impact assessment for the development. The Ministry of the Environment, Conservation and Parks (MECP) has developed additional requirements for assessment since the late 1970's including Procedure D-5-4: Technical Guideline for Individual On-site Sewage Systems: Water Quality Impact Risk Assessment (1996), and Procedure D-5-5: Technical Guideline for Private Wells: Water Supply Assessment (1996). These are the current guidelines used by MVCA and RVCA for the review of Hydrogeological Reports submitted in support of Subdivision Plan Application Approval within the County of Lanark, Ontario.

The Township's request for a hydrogeological review of the Maberly Pines Subdivision was implemented by BluMetric to meet the following objectives.

1.1 OBJECTIVES

The objectives of this study as defined in the Request for Proposal (2021-PD-002) from TVT are as follows:

- Determine if there is sufficient groundwater available at the subdivision for development as residential lots.
- Determine if the groundwater at the subdivision is potable and of acceptable water quality.
- Determine if the hydrogeological features at the subdivision will allow development on all of the lots with sufficient capacity to support the installation of septic systems.



- Produce two conceptual lot layout plans identifying the recommended locations of wells, septic systems and dwellings based inferred groundwater flow direction and site constraints:
 - Conventional lot layout plan (Figure 3) is intended to meet “as closely as possible” the current Ministry of the Environment Conservation and Parks (MECP) regulations (i.e., Procedures D-5-4 and D-5-5) that would be required if the subdivision was developed using conventional Class 4 sewage systems.
 - Restricted lot layout plan (Figure 4) introduces measures to address the lot constraints on the private servicing, to mitigate potential impacts to well water quality.

1.2 SITE DESCRIPTION

The Maberly Pines Estate (referred to herein as “the site”) encompasses a total area of approximately 76.8 hectares and is comprised of undulating terrain (see Figures 2 and 3 for topographic contours at the site and surrounding lands) that includes bedrock ridges with interspersed lowland areas, and ponds. Existing development at the subdivision includes several access roads and residences on some of the lots. Most of the subdivision is forested land. Surrounding land uses within 500 m of the site include forested areas, cottages and some rural residences, lakes, and Bolingbroke Road. All existing development in the area utilizes private individual water supply and individual septic sewer systems as municipal servicing is not available.

1.3 SUBDIVISION

The Maberly Pines subdivision was created by a developer identified as ‘Lakeside Living’ (no longer operating) and was approved by the Provincial Government in 1980. The subdivision includes 56 lots as indicated on Figures 3 and 4 (Conceptual Lot Development Plans) and the topographic survey dated 1980 (Appendix B). The status of development of the lots is as follows:

- Five developed lots
 - One permanent residence (Lot 20)
 - Four seasonal residences (Lots 6, 24, 47, and 55)
- Two permitted lots (Lots 23 and 35)
- Vacant lots as per By-Law NO. 2021-033, Plan 21
 - Lots 2, 3, 4, 5, 7, 8, 9, 10, 11, 12, 13, 14, 15, 16, 17, 18, 19, 21, 22, 25, 26, 27, 28, 29, 30, 31, 32, 33, 34, 36, 37, 38, 39, 40, 41, 42, 43, 44, 45, 46, 48, 49, 50, 51, 52, 53, 54, 56

The minimum lot size at the subdivision is 4700 m² (0.5 hectares). The maximum lot size is 3.2 hectares and the average lot size is 1.1 hectares.



Currently, developed lots of the site are serviced by individual on-site wells and individual on-site wastewater treatment systems (OWTS). This form of servicing is consistent with the established hierarchy prescribed in the Ontario Provincial Policy Statement and is consistent with the established neighbouring rural estate lot developments.

2. METHODOLOGY

2.1 BACKGROUND INFORMATION REVIEW

A review of information pertaining to the site was conducted, including the following elements:

- Water well records from the Ontario water well information system (W/WIS) database.
- Geological information from the Ontario Geological Survey online databases.
- A report titled 'Maberly Pines Subdivision, Terrain, Hydrogeological and Ecological Analysis' (WESA, 1979), including:
 - Topographic survey of the site conducted by Geo. W. Bracken Ltd. of Smiths Falls Ontario in 1980.
 - Terrain analysis data and grain size analysis.

2.2 GROUNDWATER SAMPLING AND ANALYSIS

BluMetric contacted TVT to help identify existing well owners at the Maberly Pines Development to determine viable groundwater sampling locations and to identify recently installed wells on and within 500 m of the subdivision boundaries.

Groundwater sampling took place on November 23, 2021, and consisted of sampling one previously established location. Attempts were made to sample other locations but since they are seasonal cottages, they had already been winterized, and were vacant at the time of the site visit. In some cases, there does not appear to be an established well on the property for example, lot 47 is developed; however, there was no identification of a well in Ontario Well Records and in speaking with neighbors it was mentioned that there is no well at lot 47.

All groundwater samples were submitted for comprehensive testing of bacteriological, chemical and physical water quality parameters consistent with standard 'Subdivision Water Supply' suite of parameters in accordance with Ministry of the Environment (MOE) Regulation D-5-5. The groundwater samples were also submitted for analysis of 'RVCA recommended' Ontario Regulation 153/4 Table 8 standards for metals' including uranium, strontium, and barium, and for volatile organic compounds (VOCs).



All samples were collected unfiltered and unchlorinated and were placed directly into clean bottles supplied by the analytical laboratory. Samples were placed immediately into a cooler with ice and were transported directly to the Caduceon laboratory in Kingston. All samples were received by the laboratory within six hours of collection. Caduceon is fully accredited by the Canadian Association for Laboratory Accreditation (CALA).

2.3 OFFSITE WELL OWNER INTERVIEWS

An effort was made to interview well owners about their well and septic systems. A standard form was used to conduct each brief interview. The form includes standard questions about the well location, water quality, water quantity and potential environmental concerns. Well owner interview log sheets are included in Appendix 4.

2.4 DEVELOPMENT OF CONCEPTUAL SITE PLANS

An assessment of the suitability for development of each lot was conducted. Two development scenarios were considered, including:

- Lot layout and servicing using conventional septic systems.
- Lot layout and servicing using a combination of conventional and alternative septic systems.

3. GEOLOGY AND HYDROGEOLOGY

3.1 GEOLOGY

Surficial geological mapping information from the Ontario Geological Survey (OGS) indicates the site has “bedrock drift over Precambrian terrain”. The Soil Survey of Lanark classifies surficial soils at the site as sandy loam coded MSL-R B5-S4. Site reconnaissance by WESA in 1979 identified surficial soils as a glacial till ground moraine covering much of the area, that ranged in thickness from 0.3 metres to greater than 1 metre. The till is characterized as a non-homogeneous veneer of angular granitic pebbles and cobbles in a silty sand matrix that is discontinuous across the site (WESA, 1979). WESA further described areas of poorly stratified pebbly sand up to 5 metres in thickness. Much of the site is comprised of exposed bedrock escarpments, ridges, and knobs. A terrain map was included with the WESA (1979) report and is provided in Appendix C of this report (this information is included in Figure 3, Conceptual Lot Development Plan A – Conventional Private Services).

WESA (1979) submitted one soil sample thought to be characteristic of the site to JD Paterson of Ottawa for grain size analysis. The grain size analysis is included in the WESA (1979) report. The



soil is described by WESA as 'glacial till ground moraine'. The permeability of the soil sample was determined using the Falling Head Permeameter method to be 2.42×10^{-4} cm/sec = 68.87 min/cm.

Bedrock geology mapping information from the OGS shows that the site is in the Central Grenville Metasedimentary belt of the Precambrian Canadian Shield. The bedrock units are Neo–Mesoproterozoic (0.542 Ga to 1.6 Ga) mafic to ultramafic plutonic rocks that have undergone amphibolite prograde metamorphism (Markley et al., 2018). The metamorphic protoliths within this unit include diorite, gabbro, peridotite, pyroxenite, and anorthosite. This unit is bound to the north and south by Neo–Mesoproterozoic late felsic plutonic rocks.

3.2 HYDROGEOLOGY

In the Tay Valley Township area, the most important water supply 'aquifers' typically occur within the Precambrian bedrock. Permeability within these strata is controlled by fractures (i.e. flow is not considered to be within a 'porous media') and aquifer conditions are heterogenous.

The site is geographically situated between the highest point of the Rideau Watershed (Carnahan Lake) and the Ottawa River where an elevation change of 204 metres distinguishes the modal groundwater flow direction to be to the north/northeast (RVCA, 2021). The direction of groundwater flow in the bedrock aquifer beneath the site is interpreted to be to the northeast. Topography fluctuates in the area thereby causing groundwater recharge pathways to flow in directions dictated by slope orientations and topographic lowlands as depicted in WESA 1979. To further this interpretation, the Site is situated along the central and northern aspect of a peninsular topographic high with lowlands to the north, east, and south leading to the likelihood that locally, radial groundwater recharge flow directions occur.

3.3 WATER WELL RECORDS

A review of available MECP Water Well Records in the vicinity of the site was undertaken as part this study. This information was compared to water well records collected and reviewed by WESA in 1979.

A total of 17 MECP water well records were identified within 500 m of the subdivision, as indicated on Figure 2. Overburden thickness, depth of casing, aquifer interception points and well yield related information were reviewed in detail and included in a summary table (Table 1).



Table 1: MECP Water Well Records Summary

Well ID	Well Depth (mbgs)	Overburden Depth (m)	Casing Depth (mbgs)	Depth to Groundwater (mbgs)	Static Water Level (mbgs)	1hr Test Pump Rate (L/min)	Drawdown (m)	Specific Capacity ((L/min)/m)	Date Drilled
3503579	24.38	2.4	6.7	22.50	5.48	91.0	0.0	high	10-Sep-1973
3506287	45.72	5.5	7.0	43.28	2.74	9.0	15.5	0.6	8-Jun-1981
3506755	68.58	2.7	6.7	64.00	4.88	9.0	36.6	0.2	28-Jun-1983
3506756	19.51	0.9	6.7	18.28	5.48	18.0	0.0	high	27-Jun-1983
3506757	68.58	0.6	7.0	64.00	6.70	4.5	43.3	0.1	24-Jun-1983
3507365	56.39	1.2	6.7	44.80, 55.47	9.75	18.0	37.5	0.5	9-Sep-1985
3507887	26.52	1.8	6.7	N/A	4.57	31.8	0.0	high	14-May-1987
3509525	60.96	4.6	6.7	59.44	5.49	13.6	16.5	0.8	28-Aug-1990
3510061	49.68	1.8	6.7	48.16	18.28	36.4	27.4	1.3	29-Oct-1991
3510138	48.16	1.2	6.7	38.40, 46.33	6.10	36.4	33.5	1.1	13-Nov-1991
3513257	60.96	0.5	6.7	21.34	9.45	18.2	0.0	high	1-Mar-2001
3513786	67.06	1.8	6.7	28.04, 71.63	4.88	13.6	8.8	1.5	18-Jul-2002
3514498	42.67	2.1	6.7	15.24	7.92	18.2	3.0	6.1	23-Mar-2004
7046732	67.06	1.5	6.7	64.62	10.15	30.0	8.8	3.4	28-Jun-2007
7048408	35.05	0.9	6.7	32.92	10.60	45.0	15.2	3.0	4-Jul-2007
7158460	42.70	0.6	6.1	12.00, 36.00	7.30	27.0	4.1	6.6	25-Nov-2010
7189149	121.92	1.2	6.7	N/A	6.40	13.6	36.9	0.4	24-Sep-2012

The suitability of the aquifer to supply the proposed development was assessed using the methodology provided in MECP Procedure D-5-5 (MOEE, 1996), which indicates the number of people per house is the number of bedrooms plus one. For the purpose of this study, it is assumed that new houses in the subdivision will be four-bedroom single family homes, so the number of persons per house will be five (5). Procedure D-5-5 indicates the minimum 'per-person water requirement' is 450 L/day, which is 2,250 L/day per house (or per well). Procedure D-5-5 indicates that 'peak demand' is assumed to occur over a 120-minute period and is to be based on a per person usage rate of 3.75 L/min during that period. Using this information, the 'peak demand rate' per house is $3.75 \times 5 = 18.75$ L/min.

The Canadian Mortgage and Housing Corporation's Household Guide to Water Efficiency (CMHC, 2000, revised 2014) indicates that the average daily residential water use in Ontario is 225 L per person per day (1,125 L/day for a four-bedroom house). Current Ontario Building Code requirements (OBC, 2012) for water conservation specify that toilet and shower consumption must now comply with lower use requirements (OBC Table 7.6.4.2.A & B and Table 7.6.4.1). Based on the new requirements, toilet water demand is assumed to be 4.8 L/flush. Shower consumption is assumed to be 7.6 L/min. Toilet use accounts for approximately 25% of total domestic water use, and shower use accounts for approximately 20% (CMHC, 2014). The OBC efficiencies will result in an average per person domestic water usage of 163 L/day. This suggests that the daily household water demand could often be less than 1,000 L/day.



The review of water well records within 500 m of the subdivision provided the following information regarding water quantity:

- Four wells did not show any appreciable drawdown in response to pumping and are considered high yield.
- 10 wells were pumped at a rate that was at or near the 'peak demand rate' as indicated above with varying amounts of drawdown.
- 12 of the 17 wells appear to have sufficient yield to meet peak demand and average daily residential use requirements.

Due to the nature of the fractured bedrock aquifer at the site, a small percentage of future wells may not intersect fracture networks that will provide sufficient yield for normal residential use. For lots with low yielding wells additional water storage at surface may be required to meet peak demand requirements.

3.5 WATER QUALITY

Local groundwater quality was evaluated through the collection of samples from the water supply wells located at the site, specifically from 2003 Pond Lane (see Figure 2).

The selected parameters for analysis followed the groundwater quality parameters table listed in the Appendix C of Procedure D-5-5 and 'RVCA recommended Ontario Regulation 153/04 metals, strontium, and volatile organic compounds (VOCs). The groundwater quality analytical results are summarized in Table 2 in comparison to the available standards and objectives listed in the Ontario Drinking Water Standards, Objectives, and Guidelines (ODWSOG), Ontario Regulation 169/03, as amended, Ontario Regulation 373/15, Ontario Regulation 457/16, and Ontario Regulation 153/04 Table 8 standards.

All results for VOC parameters were below the method detection limits for each parameter (i.e. all results were non detectible). A review of the analytical data summarized in Table 2 indicates that all tested water quality parameters were below the health and aesthetic related ODWSOG except for the following:

- Hardness

The operational guideline (OG) for hardness and the aesthetic objective (AO) for total dissolved solids were exceeded for the well water sample. Sodium was detected below the AO of 200 mg/L and above the recommended notification limit of 20 mg/L for persons on a sodium reduced diet. Each of these water quality parameters are discussed further below:



Hardness - Hardness was reported at 213 mg/L, exceeding the OG of 80-100 mg/L. Hardness is caused by dissolved calcium and magnesium, and is expressed as the equivalent quantity of calcium carbonate. On heating, hard water tends to form scale deposits and can form excessive scum with regular soaps. However, certain detergents are largely unaffected by hardness. Hardness levels below 500 mg/L in drinking water are considered generally acceptable for most domestic purposes and can be treated using a conventional water softener system. Softening using a domestic water softener increases the sodium level in drinking water. Sodium concentrations are reported at 28.7 whereby a concentration exceeding 20 mg/L is to be reported to the local Medical Officer of health so that this information can be communicated to local physicians for their use with patients on sodium restricted diets.

Overall, it is BluMetric's professional opinion based on the water sampling completed that a water supply of adequate water quality is available from the local bedrock aquifer.

Regional groundwater quality was also assessed for supply wells sampled as part of the Ontario Geological Survey study, 'Ambient Groundwater Geochemistry Data for Southern Ontario, 2007–2014; Ontario Geological Survey, Miscellaneous Release—Data 283—Revised.' One well location was identified in this study within 10 km from the subject properties. 13-AG-002 is located 5.2 km east of the site. The water quality results for these locations are summarized in Table 3 in comparison to the ODWSOG. No parameters reported for the Ambient Groundwater Quality Summary samples included in Table 3 were reported in exceedance of the ODWSOG objectives.

The results from the regional well water quality assessment indicate that a well water supply of acceptable water quality is available. No concern regarding regional groundwater quality within the area of the proposed severance properties were identified.

3.6 HYDROGEOLOGICAL SENSITIVITY

The terrain analysis plan (Appendix C) shows that surficial soil thickness varies significantly across the site and includes areas of exposed bedrock. Low permeability rates of surficial soils were determined by WESA in 1979 with T-times of 68.87 min/cm, a result that comes from one amalgamated sample thought to be characteristic of the till at the site. A low percolation rate reduces the likelihood of filtration but rather promotes the transport of contaminants along surficial water flow pathways into surface water bodies.

The site is considered potentially hydrogeologically sensitive due to the generally thin layer of soil cover over bedrock in some areas. Discontinuous thin soil coverage reduces the potential for filtration of contaminants. Measures for well construction and septic system design are provided herein to mitigate potential well water quality impacts.



The subdivision does not occur within a zone that has been identified by OGS as “potentially karstic”, and no karst related features have been identified at the site.

Some degree of isolation can be inferred, based on water well record information. The aquifer at the site, which can be said to occur as a ‘hydro-stratigraphic fracture zone’ within the Precambrian bedrock unit, is encountered at depths of greater than 10 m below ground surface (i.e. water bearing fractures within the bedrock unit were identified during drilling and occur at depth >10 m. This suggests that the upper bedrock zone provides a measure of protection of the deeper water bearing fracture zones, and potentially impedes the infiltration of potentially contaminated water from the surface and in the overburden unit.

3.7 FUTURE WELL CONSTRUCTION

New lots in the subdivision will be serviced by individual drilled water supply wells completed in the Precambrian bedrock. The wells must be installed by a licensed well contractor in accordance with Ontario Regulation 903. As indicated on Figure 3 and Figure 4, water supply wells must be constructed up gradient of the septic system location(s). A minimum of 50 m separation distance between water supply wells and septic systems is prescribed as best practice by RVCA, though a 30 m separation may be acceptable to address potential hydrogeologically sensitive conditions. Furthermore, the water supply well should be located with a minimum 30 m separation distance from all water courses, as indicated as best practice by the RVCA.

Based on the review of the MECP water well records, it is apparent that well yields of 18.75 L/min or greater are available from the granite bedrock aquifer (sufficient for a 4-bedroom home) situated at depths between 40 and 100 m. As a measure to address potentially sensitive hydrogeological conditions due to shallow bedrock it is recommended that the steel water well casing be installed and grouted into place to a depth of 4 m (12 feet) into competent bedrock or to a minimum depth of 12 m, whichever depth is greatest.

The annular space between the well casing and the drilled hole should be sealed with high early strength cement grout, prepared with 4% bentonite. The objective of the procedure is to prevent contamination of the bedrock aquifer via infiltration through the well annular space. Further to this, O. Reg. 903 requires the well contractor to install a suitable sealant around the base

of the well casing where it intersects the bedrock. The following recommendations provide additional measures to ensure water supply wells are protected from surface derived contaminants:

- a. A 25.4 cm (10 in.) diameter hole should be drilled through the overburden and at least 4m (12 ft.) into competent (i.e. un-weathered) bedrock or to a minimum depth of 12 m, whichever depth is greatest.



- b. New steel casing, 15.3 cm (6 in.) in diameter, should be installed in the drilled hole. Steel casing must extend 4 m (12 feet) into competent bedrock or to a minimum depth of 12 m, whichever depth is greatest.
- c. Ontario Reg. 903 well placement requirements and grouting procedures should be followed to ensure that surface derived contaminants cannot enter the well.
- d. When the grout has set (24 hours for quick set cement, 72 hours for a regular cement), drilling can continue until a suitable water supply is obtained. The cement must be dry before drilling is continued. Disturbance of partially set grout can either cause fractures to form in the grout, or separation between the grout and the bounding materials. This could create a pathway for contamination between the overburden and the bedrock aquifer.
- e. The well must be completed with a vented and vermin proof well cap and the well casing must extend at least 40 cm (16 inches) above ground surface. The ground surface must be graded away from the well.

It is recommended that the newly constructed wells be pumped for a minimum of 6 hours after construction to ensure adequate well development and to reduce groundwater turbidity to acceptable levels before connection to the residences plumbing system.

Chlorine should be introduced at the completion of well development in sufficient quantity to produce a free chlorine residual of at least 50 mg/L (ppm). The chlorine should be mixed with the standing water in the casing using a procedure that will result in the thorough vertical mixing of the chlorine over the entire depth of the well.

The well should be completed with a submersible pump, pitless adaptor and vermin proof well cap. All such mechanical work connected to the well is to be completed by a licensed well contractor possessing a valid Class 4 pump installer's license. After completion of the mechanical work in the well, the well should be disinfected as described above.

The grading around the well casing should be slightly elevated to direct surface runoff away from the well. The casing should project approximately 400 mm above the mounded soil within 3 m in all directions from the casing.

Further to above, the installed water well must be maintained by the well owner as per the requirements under Ontario Reg. 903 (and subsequent amendments). Well maintenance requirements are provided in Chapter 11 of the MECP document, "Water Supply Wells – Requirements and Best Management Practices" (Revised April 2015) available at:

<https://www.ontario.ca/document/water-supply-wells-requirements-and-best-practices>



3.8 SURFACE STORAGE FOR LOW YIELD WELLS

The daily water usage according to Procedure D-5-5 is 2,250 L/day. The peak demand water usage is (18.75 L x 120 mins) 2,250 L in 120 minutes.

The volume of water that can be stored in each new well (based on 6" diameter well that is 70 m deep) is approximately 1,270 L. However, the pumping rate in the well may be relatively low (to ensure the water level does not draw down past the pump), so the water stored in the well will not be delivered at a fast enough rate for use during peak demand periods. For example, if the maximum sustainable flow rate from a well is 3 L/min, it can only deliver 360 L during the 120 minute long peak demand period, so an additional 1,890 L of surface storage would be required. In cases where surface storage is deemed to be necessary, the amount of surface storage should be determined based on the actual sustainable yield of the well (as determined by a suitable pumping test).

As discussed in the WESA (1979) report, development planning should preclude any high volume water usages.

3.9 POTABLE WATER TREATMENT

The water within the bedrock aquifer has elevated hardness. A standard residential grade water softener can be installed to remove hardness in the raw water. Conventional water softeners will introduce sodium into the water supply, and it may be appropriate to bypass the water softener with a separate tap for drinking water.

3.10 WASTEWATER TREATMENT AND DISPOSAL

MECP Procedure D-5-4 (Technical Guidelines for Individual On-site Sewage Systems: Water Quality Impact Risk Assessment, MOEE, 1996) provides a methodology for assessing the risks associated with individual onsite sewage systems. Procedure D-5-4 indicates that developments consisting of lots which average 1 hectare (with no lot being smaller than 0.8 hectares) may not require a detailed hydrogeological assessment if it can be demonstrated that the area is not hydrogeologically sensitive. Although the average lot size in the subdivision is 1.1 hectares, 16 of the lots are less than 0.8 hectares (see Table 5 for lot size details), so a nitrate impact assessment has been provided.

3.11 PREDICTIVE NITRATE IMPACT ASSESSMENT

In assessing the impact of the subdivision, the estimate of groundwater recharge, by infiltration from precipitation, is the primary site-specific input parameter. In this regard, assumptions are required to be made with respect to evaporation and evapotranspiration, as well as infiltration



and runoff rates. The rate of infiltration will be dependent upon surficial soil types, vegetative ground covers and their distribution, and site topography.

In conducting our assessment, a mean annual precipitation value of 939.8 mm/year was used (Environment Canada, 2021). An estimation of infiltration was calculated based on site specific information and the infiltration factors provided in the document MOEE Hydrogeological Technical Information Requirements for Land Development Applications (MOEE, 1995). A calculation is provided in Appendix 4.

The cumulative nitrate impact for this subdivision has been calculated to be 4.98 mg/L with parameters detailed in Table 4a. Nitrate concentrations in onsite and offsite wells is assumed to be typically non-detectable (see onsite groundwater analytical results in Table 2) so the additional loading will be well below the provincially mandated limit of 10 mg/L. As such the subdivision as a whole should have an acceptable impact.

3.12 SEWAGE SYSTEM DESIGN

Assuming a four bedroom residence, the septic system would have to be sized to accommodate a daily sewage flow rate (Q) of 1600 L/d (OBC Table 8.2.1.3.A). Given that the lots have soils with a percolation time (T-time) >50 min/cm or bedrock, whereby the OBC specified loading rate would be 4 L/m²/d (Table 8.7.4.1.A) the mantle loading area can thereby be calculated. The OBC further specifies that the mantle is to be constructed of suitable leaching bed fill to a depth of at least 250 mm of the loading area and extend at least 15 m beyond the outer distribution pipes in any direction in which the effluent entering the soil will be moving horizontally.

Sewage systems must be designed in accordance with Part 8 of the Ontario Building Code (OBC). The OBC sets out minimum design and construction standards for all approved classes of sewage systems. It is proposed that private services be carefully assessed based on lot size, proximity to surface water bodies, and the slope of the land.

An assessment of the suitability for development of each lot was conducted. Two development scenarios have been considered: lot servicing using conventional septic systems (see Figure 3, Conceptual Lot Development Plan A – Conventional Private Services), and lot servicing incorporating alternative septic systems (see Figure 4, Conceptual Lot Development Plan A – Restricted Private Services). Table 5 (below) provides a summary of our analysis and a breakdown of the lots in terms of development status, lot size, suitability for conventional septic system, and constraints (water bodies, steep slopes). Table 5 also provides a list of concerns where conventional systems are not deemed suitable, along with suggested solutions/recommendations for the lots that will require alternatives to conventional systems. Figure 4 reflects the following tabulated breakdown of sites according to the rationale described above.



Table 5: Lot Serviceability Summary

Lot #	Area (m ²)	Status	Conventional / Restricted	Restrictions / Considerations	Solutions / Recommendations
1	8737	Vacant	Conventional	Observatory	Raised Bed
2	13952	Vacant	Conventional	N/A	Raised Bed
3	8693	Vacant	Conventional	Gradient	Max. 4:1 slope for septic / Raised Bed
4	6824	Vacant	Conventional	N/A	Raised Bed
5	8066	Vacant	Conventional	N/A	Raised Bed
6	7712	Developed	-	-	-
7	14003	Vacant	Conventional	N/A	Raised Bed
8	12012	Vacant	Conventional	Surface water body	OBC clearances 15 metres / WESA 1979 = 30 m / Raised Bed
9	17486	Vacant	Conventional	N/A	Raised Bed
10	11369	Vacant	Conventional	N/A	Raised Bed
11	11596	Vacant	Conventional	N/A	Raised Bed
12	9687	Vacant	Conventional	N/A	Raised Bed
13	8030	Vacant	Conventional	N/A	Raised Bed
14	8075	Vacant	Conventional	Gradient	Max. 4:1 slope for septic // Raised Bed
15	9771	Vacant	Conventional	Gradient	Max. 4:1 slope for septic / Raised Bed
16	9118	Vacant	Conventional	Gradient, Exposed Bedrock	Max. 4:1 slope for septic, Raised Bed
17	10604	Vacant	Conventional	Gradient, Exposed Bedrock	Max. 4:1 slope for septic, Raised Bed
18	9068	Vacant	Restricted	Gradient, Exposed Bedrock, well proximity to neighboring wells	Max. 4:1 slope for septic, Alternative Sewage Treatment System
19	7966	Vacant	Restricted	Gradient, Exposed Bedrock, well proximity to neighboring wells	Max. 4:1 slope for septic, Alternative Sewage Treatment System
20	12424	Developed	-	-	-
21	8488	Vacant	Conventional	N/A	Raised Bed
22	7789	Vacant	Conventional	N/A	Raised Bed
23	11079	Permitted	-	-	-
24	10840	Developed	-	-	-
25	12592	Vacant	Restricted	Gradient, Exposed Bedrock	Max. 4:1 slope for septic, Raised bed
26	11831	Vacant	Conventional	N/A	Raised Bed
27	4700	Vacant	Restricted	Lot size	Alternative Sewage Treatment System
28	6974	Vacant	Conventional	Gradient	Max 4:1 slope for septic / Raised Bed
29	10770	Vacant	Conventional	N/A	Raised Bed
30	6233	Vacant	Conventional	N/A	Raised Bed
31	32459	Vacant	Conventional	N/A	Raised Bed
32	20926	Vacant	Conventional	N/A	Raised Bed
33	15389	Vacant	Conventional	Exposed Bedrock	Raised Bed
34	9840	Vacant	Conventional	Gradient	Max 4:1 slope for septic / Raised Bed
35	6145	Permitted	-	-	-
36	7609	Vacant	Conventional	Surface Water Body	OBC clearances 15 metres from surface waters / WESA 1979 = 30 m / Raised Bed
37	6791	Vacant	Restricted	Lot size, Surface Water Body, well proximity to septic treatment system, well proximity to neighboring wells	OBC clearances 15 metres from surface waters / WESA 1979 = 30 m / Recommended >30 metres between well and septic / Alternative Sewage Treatment System
38	6442	Vacant	Restricted	Lot size, Surface Water Body, well proximity to septic treatment system, well proximity to neighboring wells	OBC clearances 15 metres from surface waters / WESA 1979 = 30 m / Recommended >30 metres between well and septic / Alternative Sewage Treatment System
39	8473	Vacant	Conventional	Gradient, Surface Water Body	Max. 4:1 slope for septic, OBC clearances 15 metres
40	8287	Vacant	Conventional	N/A	Raised Bed
41	13254	Vacant	Conventional	Gradient	Max. 4:1 slope for septic
42	6413	Vacant	Conventional	N/A	Raised Bed
43	6901	Vacant	Conventional	N/A	Raised Bed
44	6522	Vacant	Restricted	Lot size, Surface Water Body	Max. 4:1 slope for septic, OBC clearances 15 metres / WESA 1979 = 30m not possible
45	9520	Vacant	Restricted	Lot size, Surface Water Body	Max. 4:1 slope for septic, OBC clearances 15 metres / WESA 1979 = 30m not possible
46	10298	Vacant	Conventional	N/A	Raised Bed
47	7389	Developed	-	-	-
48	7120	Vacant	Conventional	N/A	Raised Bed
49	12097	Vacant	Conventional	N/A	Raised Bed
50	11534	Vacant	Conventional	Exposed Bedrock	Raised Bed
51	12634	Vacant	Conventional	Exposed Bedrock	Raised Bed
52	10099	Vacant	Conventional	Gradient, Exposed Bedrock	Max. 4:1 slope for septic, Raised Bed
53	15447	Vacant	Conventional	N/A	Raised Bed
54	16997	Vacant	Conventional	Surface Water Body	OBC clearances 15 metres from surface waters / WESA 1979 = 30m / Raised Bed
55	23678	Developed	-	-	-
56	18025	Vacant	Conventional	N/A	Raised Bed



For lots that meet clearances and requirements in accordance with sections 8.2.1.6, 8.7.4.2, 8.7.5.3 of the OBC, it is suggested that traditional Class 4 sewage systems consisting of a septic tank and leaching bed be implemented (see Figure 4). For lots that exceed clearances and requirements outlined in sections 8.2.1.6, 8.7.4.2, and 8.7.5.3 of the OBC it is suggested that alternative sewage treatment systems be implemented (see Figure 4).

Section 8.7.4.2 and 8.7.5.3 of the OBC requirements state that there must be a minimum of 900 mm of suitable soil or leaching bed fill present between the base of the filter bed or absorption trenches and native soil with a T-time >50 min/cm, bedrock, or high groundwater table. All lots at the site consist of either bedrock or a varying thickness of surficial soil with a T-time of >50 min/cm thereby requiring, at a minimum, fully raised Class 4 filter beds and/or absorption trench style leaching beds for all vacant lots. For lots that have sloping topography, a 4:1 gradient (25% slope) is the maximum allowable for septic bed application.

The conventional Lot Development Plan (Figures 1) shows the size of the leaching beds required for a 4-bedroom household. The leaching beds have been located as close as possible to be in accordance with section 8.2.1.6 of the OBC. The sewage system layouts on Figure 2 are fully raised leaching beds with an imported sand mantle that covers an area of 500 sq. metres. This area was determined using an equation to determine the loading rate area required (LRAR) and is as follows:

$$LRAR = Q / T$$

Where: Q = The total daily design sewage flow in litres

T = The percolation time of the native soil in min/cm

In accordance with OBC, a minimum separation of 18 m for fully raised systems is required between a well and a Class 4 sewage system, however, it has been recommended that this distance be increased to ≥30 metres. Clearance distances also apply to wells and sewage systems located on neighbouring lots.

In all instances, careful, site-specific analysis of the soil morphology in the area of each proposed leaching bed is required during the design stages of the leaching bed in order to determine if sufficient soil exists to facilitate the use of native soil for subgrade preparation.

It is not the intent of Figure 2 (Lot Development Plan) to restrict placement of a dwelling on each lot. While the actual configuration and position of the home may change, the relative position of the home, sewage system and well should be maintained. In all cases, the separation criteria for the immediate and neighbouring lots should be followed.



Alternative Sewage System Design Considerations

Alternative sewage treatment systems can be implemented on lots that are steeply sloping, have less than 0.5 hectares of useable land, or have surface water bodies that cover more than 30% of the lot. This practice would reduce the likelihood of excess nitrates entering surface water/groundwaters. The highly variable sloping terrain and bedrock escarpments at the site bring about an added level of complexity in septic system placement on some of the lots. Surface water runoff will need to be controlled upgradient of these systems to avoid untimely erosion of septic systems and reduce the potential of an unnecessary influx of partially treated wastewater into the surrounding environment.

Oakley et al., 2010 distinguished that conventional onsite wastewater treatment systems comprised of sand filters and denitrifying bioreactors (septic tanks) are the most robust and reliable wastewater treatment methods however, their implementation is constricted to larger lot size.

As a substitute to the use of a traditional septic tank and filter beds or absorption trench style leaching beds, alternative treatment of sewage may be utilized in accordance with OBC. As depicted in Figure 3, the area required for alternative sewage treatment units greatly reduces the sewage treatment footprint allowing for lots with tight clearances to be developed. These alternative sewage treatment systems vary in technologies from porous bacterial enriched foam and denitrifying lignocellulose mediums to microbial electrochemical septic tanks (MESTs). They are classified as Class 4 sewage systems and are therefore held to the same building code in the OBC.

Alternative sewage treatment systems that are suggested for the site include but are not limited to:

- Waterloo Biofilter – Designed to perform on difficult sites including small remote lots, areas of exposed bedrock, in soils with low permeability, areas with high water tables, and environmentally sensitive areas. There are many applications to suit the needs of the site-specific conditions. Third party tested. Canadian manufactured.
- Ecoflo biofilter by Premier Tech – Designed to perform on difficult sites including small remote lots, areas of exposed bedrock, in soils with low permeability, areas with high water tables, and environmentally sensitive areas. United States manufactured.

4. CONCLUSIONS

The following statements and conclusions are based on the investigation and analysis contained within this report:



- Existing water well records within 500 m of the subdivision show that well yields are acceptable in most cases. Some wells have relatively low yields. This may be due to the nature of the fractured bedrock aquifer at the site. A small percentage of future wells may not intersect fracture networks that will provide sufficient yield for normal residential use.
- If the yield of any future well is insufficient to provide an adequate quantity of water to meet 'peak demand' requirements, surface storage may be required. In these cases, the amount of surface storage should be determined based on the actual sustainable yield of the well, as determined by a six-hour pumping test.
- Analytical results from a water quality sample obtained from the well at 2003 Pond Lane (within the subdivision) are all below the limits specified in the ODWSOG, except for the result for hardness. Hardness levels below 500 mg/L in drinking water are considered generally acceptable for most domestic purposes and can be treated using a conventional water softener system.
- The concentration of sodium (28.7 mg/L) in the sample from the well at 2003 Pond Lane exceeds 20 mg/L. The local Medical Officer of Health should be notified when the sodium concentration exceeds 20 mg/L.
- The site is potentially hydrogeologically sensitive. Discontinuous thin soil coverage reduces the potential for filtration of contaminants. Measures for well construction and septic system design are provided herein to mitigate potential water quality impacts. The results of a site wide predictive nitrate impact assessment show that additional nitrate loading from individual septic systems should be well below the provincially mandated limit of 10 mg/L so the subdivision should have an acceptable impact.
- The subject property is suitable for development as a residential subdivision at the proposed density, if future development incorporates appropriate alternatives for wastewater treatment at lots that are not suitable for conventional systems. Any potential impacts to neighbouring well users are expected to be minimal.

5. RECOMMENDATIONS

5.1 WATER SUPPLY

- All future water wells in the subdivisions should be constructed so that the steel water well casing is installed and grouted into place to a depth of 4 m (12 feet) into competent bedrock or to a minimum depth of 12 m, whichever depth is greatest. This will reduce the potential of contamination. If any new well is deemed to be incapable of providing an adequate supply or use with supplemental storage (i.e. extremely low yield), it should be decommissioned according to the requirements of O.Reg. 903.
- At the time of new well installation, the drilling of the casing hole, installation of casing, and grouting of the annular space should be inspected by a licensed Professional Engineer



or Professional Geoscientist of Ontario. All well construction must be carried out by a licensed well technician.

- Wells should be developed to a sand free state in order to ensure that the residual turbidity created by the well drilling activities is completely purged from the well. Additional well development, prior to placing the well into use, is strongly recommended in order to provide adequate development of the formation and remove extraneous rock debris from the aquifer pathways.
- All future water wells should be constructed so that the top of well casing is a minimum of 400 mm above the finished grade within a 3 m radius of the wellhead. The grade should slope away from the wellhead in all directions for a distance of at least 3 m.
- Well owners should ensure that the wellhead and surrounding area are maintained in accordance with the requirements of O.Reg. 903. Future well owners should refer to the MECP Water Supply Wells Requirements and Best Management Practices, (Revised April 2015) website at: <https://dr6j45jk9xcmk.cloudfront.net/documents/4410/a-wwbmp-title-master-table-of-contents-chapter-1.pdf>
- The raw water found in the water supply aquifer system is considered to be hard. Residential grade water softeners are recommended where water hardness is deemed unsuitable. A warning clause addressed to people on low sodium diets should be registered on title regarding the elevated concentration of sodium (> 20 mg/L) associated with water softeners.

5.2 WASTEWATER TREATMENT

- For lots that meet clearances and requirements in accordance with the OBC, it is suggested that fully raised Class 4 sewage systems consisting of a septic tank and leaching beds be implemented (i.e. conventional systems with raised beds). Imported fill should be used to raise septic beds no less than 900 mm above native ground surface. These lots are listed in Table 5 and indicated on Figure 4.
- Special attention should be taken with the placement of fully raised Class 4 sewage systems on steeply sloping lots that meet clearances and requirements in accordance with the OBC, including as indicated in Table 5 and on Figure 4 (i.e. conventional systems with raised beds on sloping lots).
- For lots that exceed clearances and requirements outlined in the OBC it is suggested that alternative sewage treatment systems be implemented. These lots are identified in Table 5 and on Figure 4 and include Lots 18, 19, 27, 37, and 38.
- Special attention should be taken with the placement of septic beds on sites with water bodies. Ideally a minimum distance of 30 m should be maintained between the water body high water line and septic bed as recommended by WESA (1979). In some cases (Lots 44 and 45) this will not be possible, so the OBC minimum distance of 15 m must be maintained. These lots are indicated in Table 5.



- A lot-specific investigation should be carried out for the detailed sewage system design and site grading plan at each lot as part of the building permit application process.
- Proposed well, septic, and building locations are noted on Figure 4 (Conceptual Lot Development Plan B – Restricted Private Services).
- Future owners of individual onsite wastewater treatment systems should familiarize themselves with basic safety and maintenance information which is available at: http://www.omafr.gov.on.ca/english/environment/facts/sep_smart.htm

The conclusions presented in this report represent our professional opinion and are based on the conditions observed on the dates set out in the report for the specific locations where samples were collected, the information available at the time this report was prepared, the scope of work, and any limiting conditions noted herein. BluMetric provides no assurances regarding changes to conditions after the time of the assessment. BluMetric makes no warranty as to the accuracy or completeness of the information provided by others or of the conclusions and recommendations predicated on the accuracy of that information.

This report has been prepared for Tay Valley Township. Any use a third party makes of this report, any reliance on the report, or decisions based upon the report, are the responsibility of those third parties unless authorization is received from BluMetric in writing. BluMetric accepts no responsibility for any loss or damages suffered by any unauthorized third party as a result of decisions made or actions taken based on this report.

This report was written by Matthew DeGeer and Russell Chown of BluMetric and reviewed by Robert Hillier of BluMetric. We trust that this assessment satisfies local requirements. If you have any questions, please do not hesitate to contact the undersigned.

Respectfully submitted,
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TABLES

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Table 2: Well Water Quality Summary
Tay Valley Township - Maberly Pines Subdivision - 56 lot development

Parameter	Units	MDL	Regulation				Sample ID: 2003-01
Sample Date			ODWSOG ^{1, 2}		LG&LDHU ³	O. Reg. 153/04 ⁶ Table 8	23-11-2021
			Objective	Type of Objective			
Field Measurements							
Conductivity	uS/cm	1	-	-	-	-	488.5
pH	pH units	0.01	-	-	-	-	6.99
Hydrogen Sulphide	-	-	-	-	-	-	No odour
Temperature	°C	0.1	-	-	-	-	8.9
Visual Clarity	-	-	-	-	-	-	Clear & Colourless
Microbiological Parameters							
E. Coli	CFU/100 mL	1	0 CFU/100 mL	MAC	0 CFU/100 mL	-	0
Fecal Coliforms	CFU/100 mL	1	-	-	-	-	0
Total Coliforms	CFU/100 mL	1	0 CFU/100 mL	MAC	5 CFU/100 mL	-	0
General Inorganics							
Alkalinity, total	mg/L	5	500 mg/L	OG	-	-	234
Ammonia as N	mg/L	0.01	-	-	-	-	0.02
Dissolved Organic Carbon	mg/L	0.5	5 mg/L	AO	-	-	Hold for results
Colour	TCU	2	5 TCU	AO	-	-	<2
Conductivity	uS/cm	5	-	-	-	-	476
Hardness	mg/L	1	80-100	OG	-	-	213
pH	pH Units	0.1	-	-	-	-	8.12
Phenolics	mg/L	0.001	-	-	-	-	-
Total Dissolved Solids	mg/L	1	500 mg/L	AO	-	-	246
Sulphide	mg/L	0.01	0.05 mg/L	AO	-	-	-
Tannin & Lignin	mg/L	0.1	-	-	-	-	-
Total Kjeldahl Nitrogen	mg/L	0.1	-	-	-	-	-
Turbidity	NTU	0.1	5 NTU	AO	-	-	0.2
Anions							
Chloride	mg/L	1	250 mg/L	AO	-	-	7.2
Fluoride	mg/L	0.1	1.5 mg/L	MAC	-	-	0.2
Nitrate as N	mg/L	0.1	10 mg/L	MAC	-	-	<0.10
Nitrite as N	mg/L	0.1	1 mg/L	MAC	-	-	<0.10
Sulphate	mg/L	1	500 mg/L	AO	-	-	18
Metals							
Antimony (Sb)	mg/L	0.0005	0.006 mg/L	IMAC	-	0.006	<0.0001
Arsenic (As)	mg/L	0.001	0.01 mg/L	IMAC	-	0.025	<0.0001
Barium (Ba)	mg/L	0.001	1 mg/L	MAC	-	1	0.086
Beryllium (Be)	mg/L	0.0005	-	-	-	0.004	<0.0001
Boron (B)	mg/L	0.01	5 mg/L	IMAC	-	5	0.153
Cadmium (Cd)	mg/L	0.0001	0.005 mg/L	MAC	-	0.0021	<0.000015
Calcium (Ca)	mg/L	0.1	-	-	-	-	55.8
Chromium (Cr)	mg/L	0.001	0.05 mg/L	MAC	-	0.05	<0.002
Cobalt (Co)	mg/L	0.0002	-	-	-	0.0038	0.0001
Copper (Cu)	mg/L	0.001	1 mg/L	AO	-	0.069	0.019
Iron (Fe)	mg/L	0.1	0.3 mg/L	AO	-	-	<0.005
Lead (Pb)	mg/L	0.0001	0.01 mg/L	MAC	-	0.01	0.00018
Magnesium (Mg)	mg/L	0.2	-	-	-	-	18
Manganese (Mn)	mg/L	0.005	0.05 mg/L	AO	-	-	0.018
Molybdenum (Mo)	mg/L	0.005	-	-	-	-	0.0008
Nickel (Ni)	mg/L	0.005	-	-	-	0.1	<0.0002
Potassium (K)	mg/L	0.1	-	-	-	-	2.8
Selenium (Se)	mg/L	0.001	0.05 mg/L	-	-	0.01	<0.001
Silver (Ag)	mg/L	0.0001	-	-	-	0.0012	<0.0001
Sodium (Na)	mg/L	0.2	200 mg/L ⁴	-	-	-	28.7
Strontium (Sr)	mg/L	0.001	7.0 ⁵	MAC	-	-	0.557
Thallium (Tl)	mg/L	0.0001	-	-	-	0.002	<0.00005
Uranium (U)	mg/L	0.0001	0.02 mg/L	-	-	0.02	0.00198
Vanadium (V)	mg/L	0.001	-	-	-	0.0062	0.0002
Zinc (Zn)	mg/L	0.01	5 mg/L	-	-	0.89	0.006
Volatile Organic Compounds							
Acetone	µg/L	30	-	-	-	2700	<30
Benzene	µg/L	0.5	1.0 µg/L	MAC	-	5	<0.5
Bromodichloromethane	µg/L	2	-	-	-	16	<2
Bromoform	µg/L	5	-	-	-	25	<5
Bromomethane	µg/L	0.5	-	-	-	0.89	<0.5
Carbon Tetrachloride	µg/L	0.2	-	-	-	0.79	<0.2
Monochlorobenzene	µg/L	0.5	-	-	-	-	<0.5
Chloroform	µg/L	1	-	-	-	2.4	<1
Dibromochloromethane	µg/L	2	-	-	-	25	<2
Dichlorobenzene, 1, 2-	µg/L	0.5	200 µg/L	MAC	-	3	<0.5
Dichlorobenzene, 1, 3-	µg/L	0.5	-	-	-	59	<0.5
Dichlorobenzene, 1, 4-	µg/L	0.5	5.0 µg/L	MAC	-	1	<0.5
Dichlorodifluoromethane	µg/L	2	-	-	-	590	<2
Dichloroethane, 1, 1-	µg/L	0.5	-	-	-	5	<0.5
Dichloroethane, 1, 2-	µg/L	0.5	5.0 µg/L	MAC	-	1.6	<0.5
Dichloroethylene, 1, 1-	µg/L	0.5	14.0 µg/L	MAC	-	1.6	<0.5

Table 2: Well Water Quality Summary
Tay Valley Township - Maberly Pines Subdivision - 56 lot development

Parameter	Units	MDL	Regulation				Sample ID: 2003-01
Sample Date			ODWSOG ^{1, 2}		LG&LDHU ³	O. Reg. 153/04 ⁶	23-11-2021
			Objective	Type of Objective		Table 8	
Dichloroethene, cis-1, 2-	µg/L	0.5	-	-		-	<0.5
Dichloroethene, trans-1, 2-	µg/L	0.5	-	-		-	<0.5
Dichloropropane, 1, 2-	µg/L	0.5	-	-		5	<0.5
Dichloropropene, cis-1,3-	µg/L	0.5	-	-		-	<0.5
Dichloropropene, trans-1,3-	µg/L	0.5	-	-		-	<0.5
Dichloropropene, trans-1,3-	µg/L	0.5	-	-		-	<0.5
Dichloropropene 1,3- cis+trans	µg/L	0.5	-	-		-	<0.5
Ethylbenzene	µg/L	0.5	140 µg/L	MAC		2.4	<0.5
Dibromoethane, 1,2- (Ethylene Dibromide)	µg/L	0.2	-	-		-	<0.2
Hexane	µg/L	5	-	-		51	<5
Methyl Ethyl Ketone	µg/L	20	-	-		1800	<20
Methyl Isobutyl Ketone	µg/L	20	-	-		640	<20
Methyl-t-butyl Ether	µg/L	2	-	-		15	<2
Dichloromethane (Methylene Chloride)	µg/L	5	-	-		50	<5
Styrene	µg/L	0.5	-	-		5.4	<0.5
Tetrachloroethane, 1, 1, 1, 2-	µg/L	0.5	-	-		1.1	<0.5
Tetrachloroethane, 1, 1, 2, 2-	µg/L	0.5	-	-		1	<0.5
Tetrachloroethylene	µg/L	0.5	10 µg/L	MAC		1.6	<0.5
Toluene	µg/L	0.5	60 µg/L	MAC		22	<0.5
Trichloroethane, 1, 1, 1-	µg/L	0.5	-	-		200	<0.5
Trichloroethane, 1, 1, 2-	µg/L	0.5	-	-		4.7	<0.5
Trichloroethylene	µg/L	0.5	5.0 µg/L	MAC		1.6	<0.5
Trichlorofluoromethane	µg/L	5	-	-		150	<5
Vinyl Chloride	µg/L	0.2	1.0 µg/L	MAC		0.5	<0.2
Xylene, m, p-	µg/L	1	-	-		-	<1
Xylene, o-	µg/L	0.5	-	-		-	<.5
Xylene, m, p, o-	µg/L	1.1	-	-		-	<1.1

Notes:

- 1 - Denotes Ontario Drinking-Water Standards Objectives and Guidelines, June 2006 (O. Reg. 169/03, as amended)
 - 2 - Denotes Ontario Drinking-Water Standards Objectives and Guidelines, June 2006 (O. Reg. 373/15, s. 1; O. Reg. 457/16, s. 1.)
 - 3 - Denotes Leeds, Grenville, and Lanark District Health Unit Safe Drinking Water Interpretation
 - 4 - Potential elevated sodium intake should be communicated to your physician when concentration exceeds 20 mg/L.
 - 5 - Health Canada Proposed Standard
 - 6 - O. Reg. 153/04 - Table 8: Generic Site Condition Standards for Use within 30 m of a Water Body in a Potable Groundwater Condition
- MAC - Maximum Acceptable Concentration
IMAC - Interim Maximum Acceptable Concentration
AO - Aesthetic Objective
OG - Operational Guideline (for water treatment)
- Denotes exceeds ODWSOG
- Denotes exceeds O. Reg. 153/04
- No standard value or parameter not tested

Table 3: Well Water Quality (Ambient Groundwater Study)
Ontario Geological Survey Ambient Groundwater Quality Summary
Tay Valley Township - Maberly Pines Subdivision - 56 lot development

Parameter	Units	Regulation		13-AG-002
		Proximity to subject property		5.02 km east
		Well Depth (m)		50.29
		ODWSOG ¹		25-May-13
Sample Date	Objective	Type of Objective		
General Inorganics				
Alkalinity, total	mg/L	500 mg/L	OG	-
Ammonia as N	mg/L	-	-	-
Colour	TCU	5 TCU	AO	-
Conductivity	uS/cm	-	-	582
DOC	mg/L	5	AO	2.2
pH	pH Units	6.5 - 8.5	-	6.27
TDS	mg/L	500	AO	387.4
Organic Nitrogen	mg/L	-	-	0.07
TKN	mg/L	-	-	0.07
Anions				
Chloride	mg/L	250 mg/L	AO	54.28
Fluoride	mg/L	1.5 mg/L	MAC	0.107
Nitrate as N	mg/L	10 mg/L	MAC	1.34
Nitrite as N	mg/L	1 mg/L	MAC	<0.005
Sulphide	mg/L	0.05	AO	NA
Sulphate	mg/L	500 mg/L	AO	50.06
Metals				
Calcium	mg/L	-	-	71.31
Iron	mg/L	0.30 mg/L	AO	<0.015
Magnesium	mg/L	-	-	12.457
Manganese	mg/L	0.05 mg/L	AO	0.002
Potassium	mg/L	-	-	3.29
Sodium	mg/L	200 mg/L	AO	13.184
Antimony (Sb)	µg/L	6	IMAC	0.015
Arsenic (As)	µg/L	25	IMAC	0.088
Barium (Ba)	µg/L	1000	MAC	84.5
Boron (B)	µg/L	5000	IMAC	74
Cadmium (Cd)	µg/L	5	MAC	<0.01
Chromium (Cr)	µg/L	50	MAC	<0.02
Copper (Cu)	µg/L	1000	AO	5.91
Lead (Pb)	µg/L	10	MAC	0.1102
Mercury (Hg)	µg/L	1	MAC	<0.0015
Selenium	µg/L	10	MAC	<0.2
Zinc	µg/L	5000	AO	1.3
Uranium	µg/L	20	MAC	1.09

Notes:

1 - Denotes Ontario Drinking-Water Standards Objectives and Guidelines, June 2006 (O. Reg. 169/03, as amended)

MAC - Maximum Acceptable Concentration

IMAC - Interim Maximum Acceptable Concentration

AO - Aesthetic Objective

OG - Operational Guideline (for water treatment)

- Denotes exceeds respective Ontario Drinking Water Standard

- No standard value or parameter not tested

Table 4 - Nitrate Attenuation from Conventional Septic Systems - Maberly Pines - All 56 Lots

Parameters		Units	Information Source/Calculation
Annual Precipitation (P) =	0.9398	m/year	Canadian Climate Normals 1981-2010 Station Data, Godfrey Station.
Runoff (Q) =	16.5	%	Silty Loam. - Table 3.1: Hydrologic Cycle Component Values, (MOE Stormwater Mangement Planning and Design Manual, March 2003)
Evapotranspiration(E) =	58.6	%	
Infiltration/groundwater recharge (R) =	24.9	%	
Concentration of Nitrate =	40	mg/L	Values as specified in Section 5.6.2 of Procedure D-5-4 Technical Guideline For Individual On-Site Sewage Sytems: Water Quality Impact Risk Assessment, Last Revision August 1996
Concentration of Nitrate =	40000	mg/m3	
Daily Flow rate of Sewage =	1000	L /day	
Number of units =	56	m ³ /year m ² m ²	Total Development Area Stormwater Management Report prepared by Forefront Engineering Inc.
Yearly Sewage Volume (S _y) =	20440		
Total Property Surface Area =	767579		
% Pervious Area =	80%		
Infiltration Surface Area (A _d) =	614063.2		
<u>Infiltration Flux, R</u>	143697.05	m ³ /year	R=P-Q-E
Yearly Volume of Sewage (S _y) =	20440	m ³ /year	M _y /(S _y + R)
Yearly Volume of Infiltration (R) =	143697	m ³ /year	
Yearly Mass Loading of Nitrate (M _y) =	817600000	mg/year	
Nitrate Conc.at Downgradient Prop.Boundary =	4981.20	mg/m ³	
" =	4.98	mg/L	
Background Nitrate Concentration =	0.00	mg/L	Max. concentration measured at test wells (2003-01)
Nitrate Total =	4.98	mg/L	

FIGURES

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LEGEND

Maberly Pines Subdivision

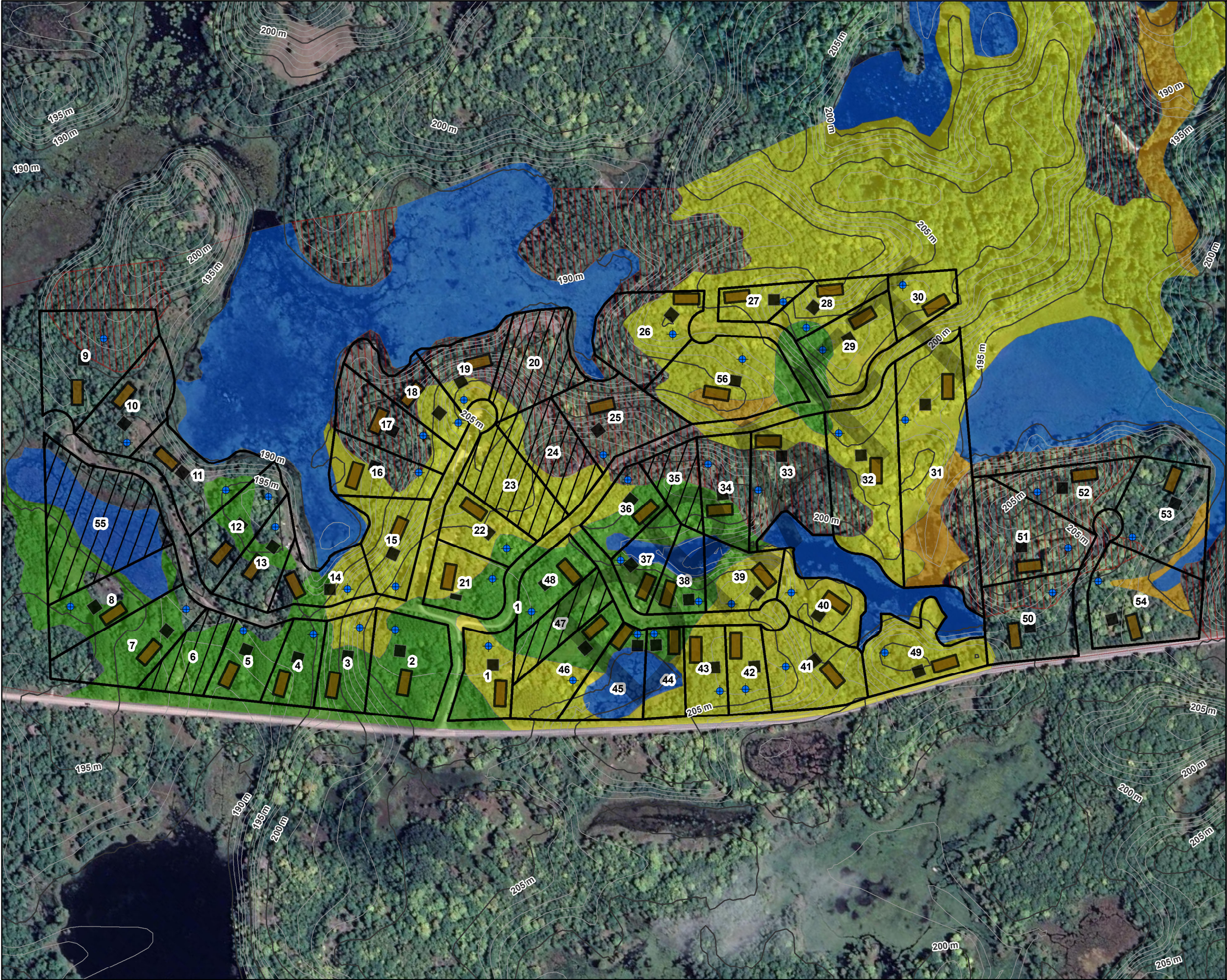
1				
REV.	DESCRIPTION	YY/MM/DD	BY	CHK
<p>REFERENCES</p> <p>PROPRIETARY INFORMATION MAY NOT BE REPRODUCED OR DIVULGED WITHOUT PRIOR WRITTEN CONSENT OF BLUMETRIC ENVIRONMENTAL INC. DO NOT SCALE DRAWING. THIS DRAWING MAY HAVE BEEN REDUCED. ALL SCALE NOTATIONS INDICATED ARE BASED ON 11"x17" FORMAT DRAWINGS.</p> <div><div>048 Kilometers</div><div>1:150,000</div><div><div>N</div><div>W</div><div>E</div><div>S</div></div></div>				
<p>CLIENT</p> <p>Tay Valley Township - Maberly Pines</p>				
<p>PROJECT</p> <p>Hydrogeology Study</p>				
<p>TITLE</p> <p>Site Location</p>				
<div><div><div><div></div><div>BluMetric™</div><div>Environmental</div></div><div><p>The Tower - The Woolen Mill, 4 Cataraqui St., Kingston, Ontario K7K 1Z7 TEL: (613) 531-2725 FAX: (613) 531-1852 Email: info@blumetric.ca Web: http://www.blumetric.ca</p></div></div></div>				
PROJECT #		DATE		
220037		November 29, 2021		
DRAWN	CHECKED	FIG NO.	REV	
GM	RC	01	0	



LEGEND

- Sample Location
- Water Well Record (MECP)
- Subdivision - 500 m Buffer
- Property Parcel
- Property Parcel - Developed
- Elevation Contour - 1 m (MNR)
- Elevation Contour - 5 m (MNR)

1				
REV.	DESCRIPTION	YY/MM/DD	BY	CHK
<p>REFERENCES</p> <p>PROPRIETARY INFORMATION MAY NOT BE REPRODUCED OR DIVULGED WITHOUT PRIOR WRITTEN CONSENT OF BLUMETRIC ENVIRONMENTAL INC. DO NOT SCALE DRAWING. THIS DRAWING MAY HAVE BEEN REDUCED. ALL SCALE NOTATIONS INDICATED ARE BASED ON 11"x17" FORMAT DRAWINGS.</p> <div></div> <div><p>1:10,000</p></div>				
CLIENT				
Tay Valley Township - Maberly Pines				
PROJECT				
Hydrogeology Study				
TITLE				
Site Layout and Well Records				
<div><div><p>The Tower - The Woolen Mill, 4 Cataraqui St., Kingston, Ontario K7K 1Z7 TEL: (613) 531-2725 FAX: (613) 531-1852 Email: info@blumetric.ca Web: http://www.blumetric.ca</p></div></div>				
PROJECT #		DATE		
220037		November 29, 2021		
DRAWN	CHECKED	FIG NO.	REV	
GM	RC	02	0	



LEGEND

- Supply Well
- Dwelling
- Septic Bed
- Property Parcel
- Property Parcel - Developed
- Elevation Contour - 1 m (MNR)
- Elevation Contour - 5 m (MNR)
- Bedrock, Flat
- Bedrock, Sloping
- Thin Till Over Bedrock
- Thick Till and Sand Over Bedrock
- Thick Till, Poorly Drained
- Beaver Swamp Marsh

1				
REV.	DESCRIPTION	YY/MM/DD	BY	CHK

REFERENCES

PROPRIETARY INFORMATION MAY NOT BE REPRODUCED OR DIVULGED WITHOUT PRIOR WRITTEN CONSENT OF BLUMETRIC ENVIRONMENTAL INC. DO NOT SCALE DRAWING. THIS DRAWING MAY HAVE BEEN REDUCED. ALL SCALE NOTATIONS INDICATED ARE BASED ON 11"x17" FORMAT DRAWINGS.

CLIENT

Tay Valley Township - Mayberly Pines

PROJECT

Hydrogeology Study

TITLE

**Conceptual Lot Development Plan A
(Conventional Private Services)**


The Tower - The Woolen Mill,
4 Cataraqui St.,
Kingston, Ontario K7K 1Z7
TEL: (613) 531-2725
FAX: (613) 531-1852
Email: info@blumetric.ca
Web: <http://www.blumetric.ca>

PROJECT # 220037		DATE December 01, 2021	
DRAWN GM	CHECKED RC	FIG NO. 03	REV 0



LEGEND

- Supply Well
- Dwelling
- Septic Bed
- Conventional Parcel
- Developed Parcel
- Likely Exceed Maximum Allowable Gradient Listed in OBC for Septic Systems
- Alternative System Recommended
- Elevation Contour - 1 m (MNR)
- Elevation Contour - 5 m (MNR)

1				
REV.	DESCRIPTION	YY/MM/DD	BY	CHK
<p>REFERENCES</p> <p>PROPRIETARY INFORMATION MAY NOT BE REPRODUCED OR DIVULGED WITHOUT PRIOR WRITTEN CONSENT OF BLUMETRIC ENVIRONMENTAL INC. DO NOT SCALE DRAWING. THIS DRAWING MAY HAVE BEEN REDUCED. ALL SCALE NOTATIONS INDICATED ARE BASED ON 11"x17" FORMAT DRAWINGS.</p> <div></div>				
CLIENT				
Tay Valley Township - Maberly Pines				
PROJECT				
Hydrogeology Study				
TITLE				
Conceptual Lot Development Plan B (Restricted Private Services)				
		<p>The Tower - The Woolen Mill, 4 Cataraqui St., Kingston, Ontario K7K 1Z7 TEL: (613) 531-2725 FAX: (613) 531-1852 Email: info@blumetric.ca Web: http://www.blumetric.ca</p>		
PROJECT #		DATE		
220037		December 01, 2021		
DRAWN	CHECKED	FIG NO.	REV	
GM	RC	04	0	

APPENDIX A

WESA, 1979 report

DRAFT



Schedule 10
Bylaw 535
Subdivision
MABERLY PINES.

MABERLY PINES DEVELOPMENT

Terrain, Hydrogeological and Ecological Analysis

Concession V Parts of Lots 12, 13, 14, 15,

Concession VI Part of Lot 13

South Sherbrooke Township



WATER AND EARTH SCIENCE ASSOCIATES LTD.

124 O'CONNOR ST., SUITE 303, OTTAWA, ONTARIO K1P 5M9

MABERLY PINES DEVELOPMENT

Terrain, Hydrogeological and Ecological Analysis

Concession V Parts of Lots 12, 13, 14, 15

Concession VI Part of Lot 13

South Sherbrooke Township

Derek P. Smith M.Sc. FGAC

Water and Earth Science Associates Ltd.

Harold J. Parsons, Director

Al Macdonald B.Sc.

Bufo Inc.

1.0 Introduction

Water and Earth Science Associates were commissioned by Mr. Jacques Noel, President of Lakeside Living Limited to conduct an analysis of the hydrogeological, terrain and ecological conditions of a proposed seasonal residential subdivision located on Concession V (parts of Lots 12, 13, 14, 15) and Concession VI (part of Lot 13), Township of South Sherbrooke. (Figure 1)

In order to establish the suitability of the property for development on wells and septic tank systems and provide planning and environmental guidelines as dictated by terrain conditions, the following site factors were studied:

1. the distribution and lithology of bedrock and surficial materials
2. topography and drainage
3. the hydrogeological characteristics of the bedrock aquifer
4. the characteristics of terrain units and their potential to disperse and attenuate septic tank effluent, and
5. the most suitable design of well and septic tank systems.

The results of our investigations are presented in the following report.

1.1 Study Methods

First, a site reconnaissance of the property was made and pertinent published literature about the physiography, geology, ecology and hydrogeology of the Little Silver Lake area was reviewed.

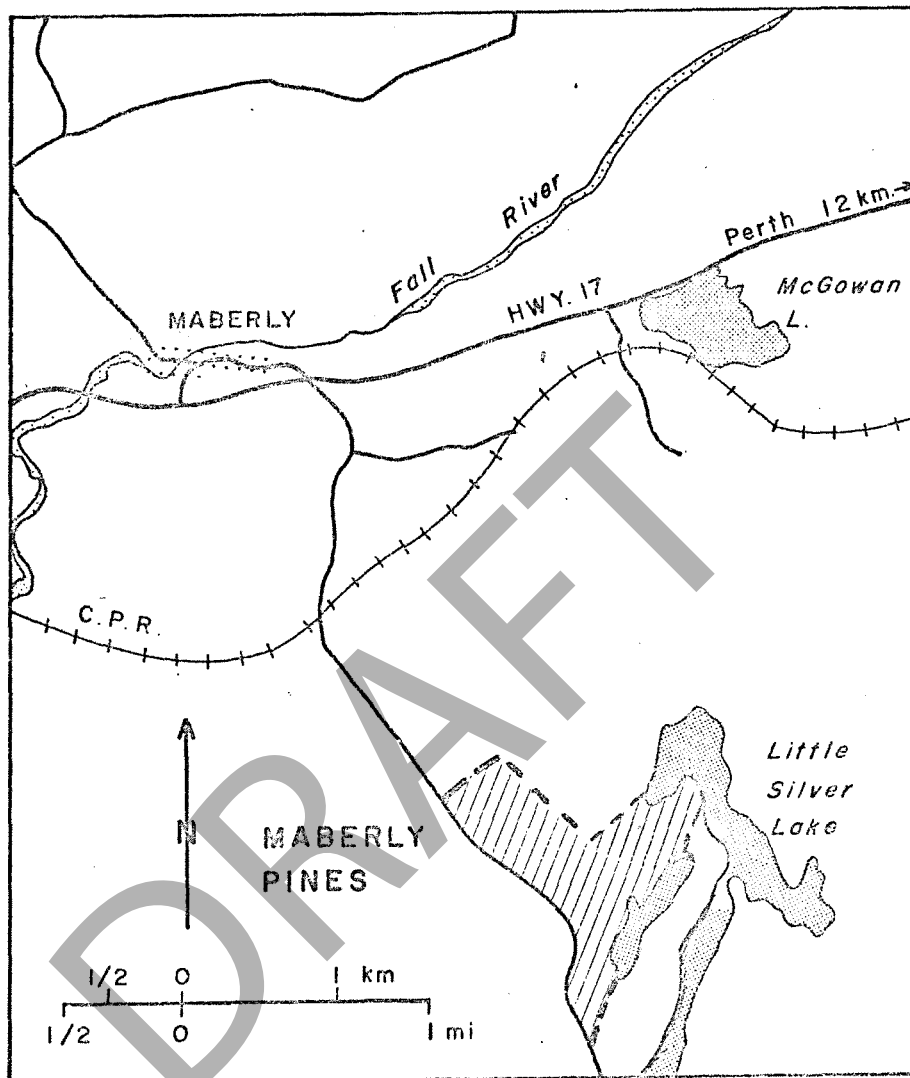


FIGURE 1
MABERLY PINES
LOCATION MAP

Then five days of field work were conducted at the site during which time the geology and ecology of the land parcel was mapped at a scale of 1:2400. Large and small scale air photographs were used during this investigation. Field mapping was conducted by geological traversing and hand digging shallow test pits into the surficial sediments.

All published well logs from Concessions 3 - 9 and Lots 11 - 16 of South Sherbrooke Township were collected and analyzed to establish the potential of aquifers within the property. The grain size distribution and hydraulic conductivity of a typical soil sample were measured in the laboratory to determine the suitability of surficial materials for the in-ground disposal of domestic sewage.

Finally, planning documents and government regulations were reviewed as a basis for the recommendations included in this report.

1.2 Physiography

Physiographically, the Maberly Pines area is made up of a series of bedrock knobs and ridges interspersed with lowland areas. The terrain has a northwest-southeast orientation which is particularly pronounced immediately south of Little Silver Lake (Figure 2). Site topography reflects the peneplanation of this region which was caused by four major glacial advances and retreats. A maximum elevation of 212 metres above sea level occurs near Little Silver Lake, although most bedrock ridges lie at 202 - 210 metres above sea level. Lowland areas occur at elevations which range from 192 to 200 metres above sea level. Some variation in the elevation of swamps occurs across the site.

For example, the large pond in the northwest corner of the site has a 192 metre water level while a small waterbody near the highway to the south of the property lies at a 200 metre elevation.

2.0 Site Geology

The Little Silver Lake area is a good example of the Precambrian Terrain which characterizes much of the Canadian Shield of Ontario and Quebec. Ancient Precambrian rocks, last deformed by the Grenville Mountain Building episode which occurred about 950 million years ago, are overlain by a thin veneer of much younger glacial and non-glacial sediments. An irregular glaciated topography with an immature drainage pattern and numerous beaver ponds in lowland areas typify this terrain type.

The geology of the Little Silver Lake site is summarized in chart form as Table 1 of this text. A brief discussion of bedrock and surficial deposits is included below. The reader is referred to the geological references cited in the bibliography of this text if more details of the geological history of the Perth-Maberly region are of interest.

2.1 Bedrock Geology

The site is underlain by a Precambrian crystalline basement complex which includes biotite gneiss, diorite, migmatite, marble, quartzite, pegmatite and related paragneissic rocks. Bedrock is foliated with a northeast - southwest trend and near vertical dips.

The upper rock surface is striated, plucked and grooved and indicates that the last movement of glacial ice across this region was

GEOLOGICAL AGE		LITHOLOGY	THICKNESS	SLOPE	GEOLOGICAL HISTORY
SURFICIAL DEPOSITS	QUATERNARY Recent Pleistocene	Soils; podzols, acidic and immature. Bog deposits, muck and peat, areas of fen vegetation, marsh.	5 to 10 cm	flat	Formed by interaction of biological, climatic and geological elements. Controlled by beaver population or formed in poorly drained lowlands, produced by high organic deposition in wet areas.
		Glacial till, angular pebbles and boulders with a silty sandy brown matrix; pebbly sand facies overlies till.	.3 m to greater than 1 metre	deposited as thin veneer on sloping bedrock	Direct deposit from glacial ice; glacial till ground moraine. Sandy facies restricted to poorly developed small drumlin structures.
BEDROCK	PRECAMBRIAN	Migmatite, biotite gneiss, diorite, marble, pegmatite and other granitized paragneisses	unknown	5 - 40% slopes, steep escarpment in places.	Eroded roots of the Grenville Mountains (950 million years old).

Table 1: Summary of Geological History

in a northeast to southwest direction. Bedrock outcrops at the ground surface throughout the property and forms abrupt bedrock escarpments in many places.

Small outcrops and escarpments are present throughout parts of the land parcel forming a rugged microrelief.

Some evidence of minor open pit feldspar mining activity is present on the property, although excavations are too small to comprise a constraint to site planning.

2.1 Surficial Geology

Bedrock is covered by a veneer of glacial till ground moraine over most of the property. The distribution of the till material and bedrock outcrop areas is shown on Figure 2 of this report.

The till ground moraine material is composed of angular granitic pebbles and cobbles with a fine sand and silt matrix. In several areas of the property, poorly stratified pebbly sand deposits are found associated with the till ground moraine. These deposits apparently range up to 5 metres in thickness, lie stratigraphically above the till material and are oriented parallel to the direction of the last ice movement. They are interpreted as being very poorly developed small drumlin structures based on this evidence. The major drumlin is located just south of the property boundary near Little Silver Lake (just outside area of Figure 2) and has been partially quarried for borrow material. Similar deposits were noticed in several areas of the site but were mapped as a sand facies of the till ground moraine material due to their diffuse form and thinness.

The composition of a typical sample of the till ground moraine material was analyzed in the laboratory with the following results:

Grain Size Distribution	Clay	2 %
	Silt	18%
	Fine Sand	36%
	Medium Sand	12%
	Coarse Sand	8%
	Gravel	24%

Permeability (using Falling
Head Permeameter) = 2.42×10^{-4} cm/sec.

Where present, the till unit is usually only a few centimetres to half a metre in thickness on ridge tops. However, in valley areas, a till thickness of 1 metre or greater was found during field investigations.

Swamp deposits include poorly drained black organic soils, muck and peat deposits. Their distribution is restricted to lowland areas and have been greatly extended in recent years by the activities of the beaver population in the area.

In general, soils formed on the sandy till ground moraine are poorly developed, are from 10 to 20 centimetres thick and have a poor potential for agricultural crop production.

3.0 Hydrogeology

In order to provide information on potential well yields and groundwater quality within the Maberly Pines subdivision, existing

well logs recorded with the Ministry of the Environment from Concessions 7, 8, 9, Lots 11 to 16 have been assembled and analyzed.

The Precambrian bedrock is the only geological unit in the study region with the potential to provide adequate quantities of groundwater for domestic water supplies. Surficial materials are too thin and discontinuous in nature to furnish reliable water sources. Therefore, dug or driven wells are considered unsuitable for use on this property.

Knowledge of the recharge characteristics, water supply potential and groundwater quality of the Precambrian aquifer is an important factor in the planning of any development of this site. A brief discussion of these points is included in the following sections.

3.1 Recharge Characteristics

Groundwater movement in the Precambrian basement rock is controlled by variations in topography between highlands and lowland areas and the pattern and extent of the fracture system present. Figure 3 illustrates in a theoretical manner how the precipitation which falls on upland recharge areas will flow downwards into the saturated groundwater zone below the water table and hence, in a lateral direction towards lowland swamp and stream discharge zones.

Saturated hydraulic gradients in Precambrian terrain are impossible to measure without detailed drilling data. Gradients in the unsaturated near-surface fracture system, however, should reflect surface topography variations and the orientation of fracture patterns closely and are typically quite high (0.2 to 0.7). Infiltration rates

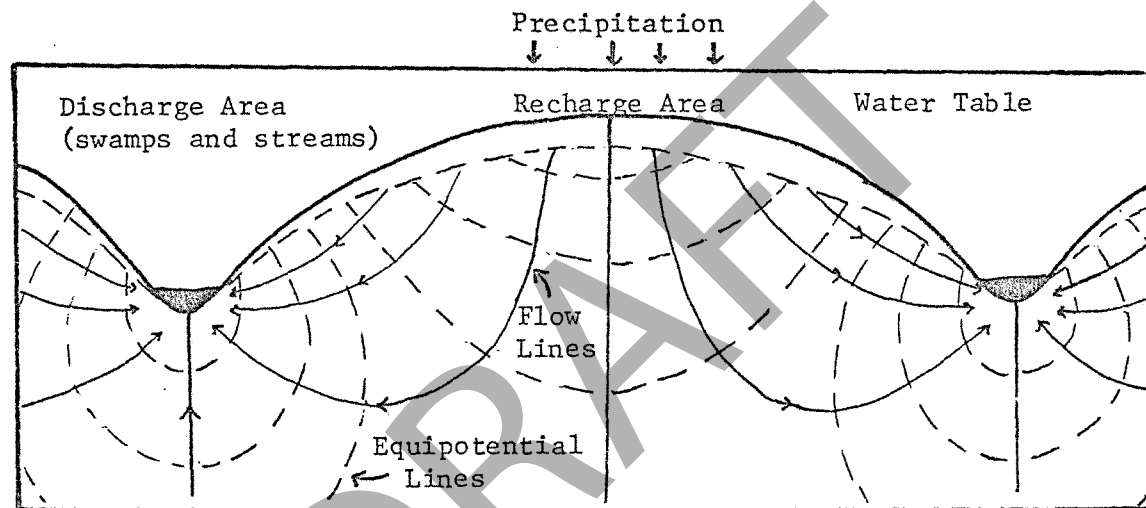


Figure 3: Idealized Model illustrating Groundwater Flow from Recharge on the Topographic Highs to Discharge in the Valleys (Hubbert, 1940)

and groundwater flow velocities should be high in this terrain but cannot be calculated because measurements of the bedrock fracture permeability have not been made. However, groundwater movement in the order of 25 - 50 metres per year is considered a reasonable estimate based on theoretical calculations.

Based on this information, wells should be located on highland areas, for two reasons:

a) septic tile weeping beds can then be located at lower elevations and will flow away from, not towards water wells

b) wells will be recharged by precipitation and will be located at a sufficient distance from lowland marsh areas to avoid drawing water from these sources. Marsh water is often of poor quality due to high organic acid concentrations, low pH or colour and odour problems.

3.2 Aquifer Potential

The water wells for all domestic wells utilizing the Precambrian bedrock aquifer in Concessions 7, 8 and 9, Lots 10 to 16 have been analyzed to provide an assessment of the groundwater supply potential in the Maberly Pines Subdivision. The 17 logs recorded with the Ministry of the Environment are included as Appendix B in this report. There is no well log information from the proposed subdivision with existing cottages along Silver Lake using lake water as a water source.

Well yields in Precambrian terrain vary as a function of the degree of fracture (i.e. fracture permeability) of the bedrock.

Well yields can vary significantly within short distances (i.e. 100 metres or less) in this rock type. It should be noted that fractures usually decrease in density with depth along the metamorphic foliation and the joint pattern in granitic rocks. Well yields are usually not significantly increased if wells are drilled beyond 50 metres as a consequence.

Water was found from 10.0 to 38.4 metres below the ground surface (mean = 21.3 metres) in these wells with a static level variation of 1.21 to 10.0 metres (mean 16.5). Well data are too sparse to permit an analysis of fracture system patterns using depth histograms. However, well depths vary from 8.2 to 35.0 metres which indicate that near surface fracture systems are supplying adequate water supplies from existing residences.

To evaluate well yields, each log was examined and classified as follows:

	Number of Wells
Poor yields (drawdowns were high, 25 - 75' after short term (1-2 hr) pump tests at 5 gpm or less)	12
Moderate yields (drawdowns were fairly low, less than 50' after short term pump tests at 5 - 10 gpm)	3
Good yields (drawdowns were low after short term pump tests at greater than 10 gpm)	2
TOTAL	17 wells

The following conclusions can be drawn from this analysis:

- a) twelve of the existing wells in this area have yields close

to the minimum required to service a domestic residence (4 igpm or 18 litres per minute). Wells should be drilled and constructed as per the recommendations outlined in Section 4.1 to maximize the well yields and eliminate potential contamination problems.

b) it is unlikely that high volume wells of 200 litres per minute or greater could be drilled on this site. Development planning should preclude high volume water usages as a consequence.

3.3 Water Quality

The water quality of groundwater from existing wells in the Little Silver Lake area is reported to be fresh, colourless and odourless. This is most likely the case on the study property.

4.0 Type of Development

It is understood that the Little Silver Lake subdivision will be a seasonal recreational development. As a consequence, septic tanks will be used primarily during summer months and water requirements will be lower than in permanent subdivisions. The recommendations proposed in this report however, are based on the assumption that some winter utilization may also occur and that conversion of dwellings to yearly use is a possibility i.e. that the development is a year-round backlot subdivision. A restriction of the subdivision to seasonal use however, should provide a large safety factor to guarantee the integrity of groundwater supplies.

4.1 Suitability for Development

Six terrain units, or land types having a unique association

of lithological, ecological and topographic characteristics have been identified on this property from our field work. These are:

1. bedrock, highly sloping
2. bedrock, flat
3. thin till over bedrock
4. thick till and sand over bedrock
5. thick till, poorly drained
6. beaver swamp

The distribution of each terrain unit is mapped on Figure 2 of this report while their characteristics are summarized as Table 2.

Terrain Unit 1 (bedrock, sloping) has little or not capability to attenuate septic tank effluent in its natural state due to the thin nature of the soil cover in these areas. High slopes, abundant outcrops and rock escarpments are major planning constraints throughout this unit. Terrain Unit 1 is not recommended for the installation of septic tank systems.

Terrain Unit 2 (bedrock, flat) has the same constraints as Unit 1 but slopes are usually less than 10% and till material is thicker in isolated pockets. Development on large lots (2 - 3 acres) is considered feasible on this unit provided tile beds are fully raised and well to septic tank spacings of 30 - 50 metres are instituted. Lot planning will require locating suitable tile bed locations first and locating dwellings second in respect to these areas.

Terrain Unit 3 and 4 are distinguished on the basis of till depth.

A typical sample of the silty sand till ground moraine gave a falling

TERRAIN UNIT	LITHOLOGY OF UNIT	THICKNESS OF SURFICIAL MATERIALS	HYDRAULIC CONDUCTIVITY	WATER TABLE DEPTH	SLOPE	SUITABILITY FOR CONVENTIONAL SEPTIC TANKS	WELL TO SEPTIC TANK SPACINGS	RECOMMENDED SEPTIC SYSTEM DESIGN
1	Bedrock, sloping, very thin veneer of till	0 - .3 m	greater than 2.43×10^{-4} cm/sec where coarse grained and thin	below bedrock surface	5 - 40% with rock escarpments	very poor, not recommended for development	-	-
2	Bedrock, flat out-crop with pockets of till	0 - 1.0 m in pockets	as below	below bedrock surface	0 - 20% rolling, rugged microrelief	poor	30 - 50 metre wells to be "upstream" from tile beds	fully raised 1 m tile beds with soil mantles
3	Thin till over bedrock	.5 - 1.5 m blanket	tested at 2.43×10^{-4} cm/sec	below bedrock surface	5 - 10%	fair to good	30 m	partially raised (.5 - 1.0 m) tile beds with soil mantles
4	Thick till and sand over bedrock	1.0 m blanket	as above	well drained, below bedrock surface	5 - 10%	excellent	30 m	septic tanks as per Ministry of Environment standards
5	Thick till poorly drained	as above	as above	within .5 m of surface	0 - 40%	poor no development	-	-
6	Beaver swamp	unknown	low	at surface	0%	nil no development	-	-

Table 2: Maberly Pines
Development Potential of Terrain Units

head permeameter reading of 2.43×10^{-4} cm/second. Table 3 summarizes published literature comparing both permeability (hydraulic conductivity) and percolation test data for different types of surficial geological materials.

Permeability is expressed as both cm/second and minutes per inch in this Table. It is impossible, however, to relate percolation times and permeability measurements directly because permeameter readings are accurate saturated flow velocity measurements done in the laboratory while percolation readings are simple field tests. Percolation tests are often highly inaccurate due to problems of stratigraphic variation, compaction and partially saturated test holes. Also, percolation tests usually give higher (i.e. more permeable) results due to the presence of temporary structures in the soil horizon (rootlets, worm burrows, fissures, cracks, thin pervious soil lenses, etc.)

The Maberly Pines till sample has a permeability of 2.43×10^{-4} cm/second (or 175 minutes per inch if percolation could be calculated directly). According to Bernhart (1972) however, this permeability would yield a field percolation test near 60 minutes/inch and would be an excellent, although slightly impervious porous media for the attenuation of septic tank effluent.

In Terrain Unit 3 and 4 where till thickness is less than 1 metre, partially raised tile beds should be required. Minimum lot sizes of 1 acre are suggested for these units.

Poorly drained till areas have been mapped as Terrain Unit 5 (Figure 2). These areas would require fill and drainage work during development and should be avoided whenever possible.

Terrain Unit 6 is swampland with no potential for development. These areas are highly sensitive ecological zones and should not be filled or altered in any manner,

Maberly
Pines
Sample

Table: Approximate Correlation of Percolation Rates
and Permeability Measurements

2.43×10^{-4} cm/sec.

Percolation Rates (x)
(Bernart 1972)

t in Minutes per inch

1 - 5

5 - 30

30 - 60

60 - 120

120 - 180

Medium
Sand

Fine Sand
to
Sand and
Silt

Loam and
Silt Soils

Clay and
Silt Soils

Heavy Clay
Soils

Correlation Based on Lithology not Calculations
(Refer to Section 4.1)

Hydraulic Conductivity
(Todd 1959)

10

1

10^{-1}

10^{-2}

10^{-3}

10^{-4}

10^{-5}

10^{-6}

10^{-7}

10^{-8}

centimetres/second

Clean
Gravel

Clean Sand, Sand/Gravel
Mixtures

Very fine sands, glacial till,
silt and clay mixtures

Unweathered
Clay

Well and septic tank design and site investigation recommendations are included in the following sections for each terrain unit.

4.1 Recommended Well Design

To minimize the risk of well water contamination and maximize well yields:

1. All wells should be drilled with a cable tool rig or an air rotary rig. Wells should be drilled slowly to minimize blockage and sealing of the fine joints and fractures in the bedrock which are the source of water in the Precambrian bedrock. In addition, wells should be surged every 5 metres during construction. Rotary drilling using "down-the-hole Hammer" technique (i.e. air percussion) seals fractures and result in low yields, over-deepened wells and high well construction costs.

2. All wells should be properly cement-grouted one casing length (about 7.5 metres) into bedrock to seal off near surface fractures close to the well which have a high potential to permit contaminated surface water from recharging the well.

3. Wells should be drilled at least 50 metres from swamps and marshes to avoid the possibility of recharging wells with poor quality water. Swamp water is often enriched in organic acids and may have an objectionable colour and odour.

4.2 Tile Field Design Recommendations

1. It is recommended that the capacity of septic tanks and the lengths of weeping tile used by increased be increased by a

factor of 1.5 over Ministry of Environment guidelines. It is felt that most septic tank systems are underdesigned for the capacity loadings placed on them by modern household appliances (e.g. dishwashers).

2. It is recommended that tile bed or well spacings within individual lots be increased to between 30 and 50 metres as a safety factor in order to minimize any risk of contamination of potable well water. Tile beds should be located blow wells to permit effluent to flow away from and not towards water supplies.

3. Septic tanks on Terrain Units 2 and 3 will require raised tile bed installations. A diagram of this design is included as Figure 4 of this report.

4. Where slopes are high (5 - 10%), tile bed construction will require:

that a 40 x 50' minimum area be infilled with semi-permeable material to reduce the slope to less than 1%

and

that a mantle of fill (20' minimum width by 2' depth) be constructed around the tile bed.

A generalized sketch of these conditions is included as Figure 5.

Tile bed construction on slopes of 10 - 25% is difficult and might require extensive remedial work with heavy construction machinery. These cases should be designed and approved on an individual basis.

5. Precambrian terrain (especially Terrain Units 1, 2 and 3) which are to be developed for seasonal and recreational uses, have a

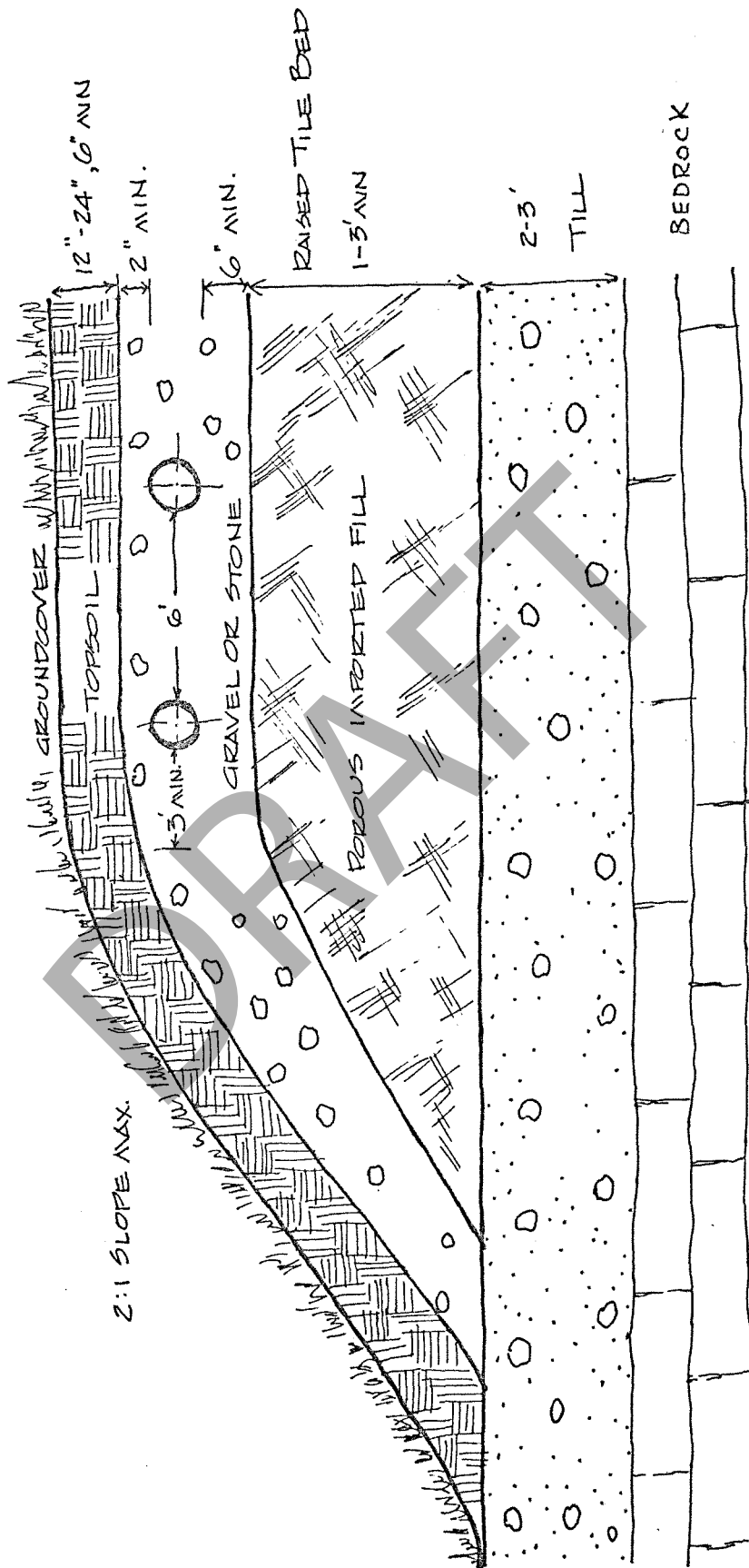
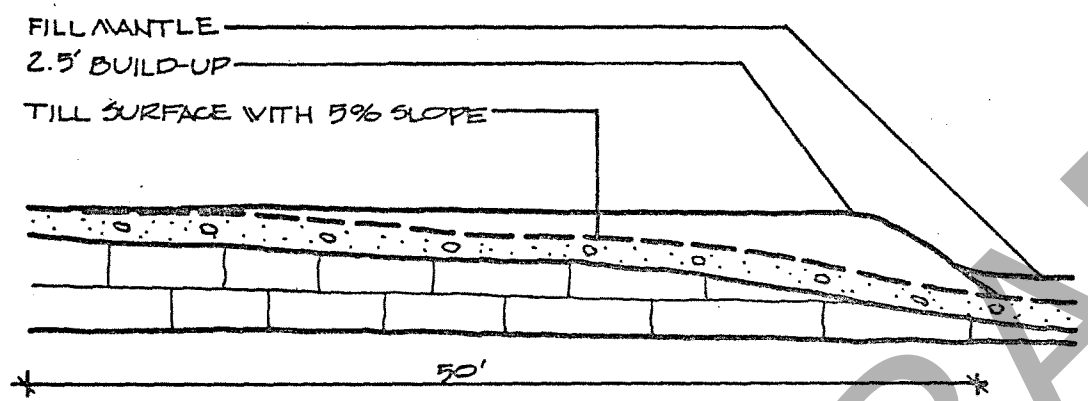
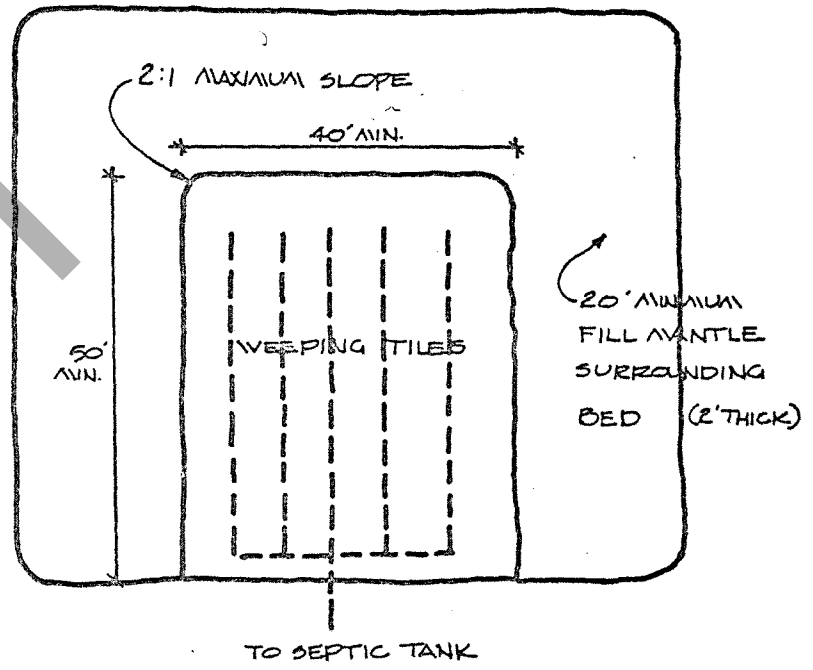


FIGURE 4 RECOMMENDED DESIGN OF LEACHING BED WHERE THIN TILL UNIT IS PRESENT

FIGURE 5: RECOMMENDED SEPTIC TANK TILE BED DESIGN ON SLOPING TERRAIN
(10% MAX. SLOPE)



CROSS SECTION SCALE: 1"=10'



PLAN VIEW SCALE 1"=20'

high potential to be serviced with Humus toilets (or some other alternative sewage system brand which does not utilize in-ground disposal methods). These toilets are functional, economical and eliminate all risk of groundwater pollution.

4.3 Site Inspections

It is recommended that a lot by lot field survey of potential tile field locations be made upon completion of the concept plan with officials of the Public Health Unit, Perth Ontario.

Any possible problems with tile bed sitings due to localized drainage channels, minor escarpments or soil thickness variations, would be identified at this time. In addition, any inaccuracies in the base map or contours which might effect site layouts would be verified at this time.

Please note that this is not a lengthy procedure but has recently become a general requirement of the Ministry of the Environment for this type of terrain.

5.0 Vegetation and Wildlife

Methodology and Format of Ecosystem Analysis

To assess the vegetation and wildlife components for ecological constraints to development, the site was divided into natural ecosystems. An ecosystem can be defined as the interaction and interdependence of all physical and biological components of any area. The physical and vegetation parameters of an individual ecosystem constitute a biotope. For this discussion, the study site has been categorized into upland biotope, lowland biotope, open field biotope

marsh and swamp biotope and lake and shoreline biotope. Each biotope is described under the following headings:

- description and distribution
- threatened species or unique associations
- species of economic importance
- constraints to development

The vegetation component of each biotope is described with regards to species composition and distribution. The discussion of unique associations at particular sites includes consideration of abundance of species and significance of the association of plants and animals to the biotope. Decisions concerning the presence of rare and endangered species are based upon each species' range, the occurrence of suitable habitat, and records in the scientific literature. Species of economic importance include game species of birds and animals, sport fishes, fur-bearers and commercial forest tree speices. Canada Land Inventory capability maps for ungulate, waterfowl and forestry production are referred to where applicable. Constraints to development were derived after evaluating sensitivities of the ecosystems to the types of disturbance generated by an estate lot housing project. Areas of high and moderate sensitivity have been mapped on Figure 2 of this report as a guideline for subdivision planning.

5.1 Upland Biotope (Terrain Units 1 and 2)

A. Description and Distribution

The upland biotope is composed of high, well-drained areas that may be forested or shrub covered and partially bare. The forested portions of upland sites are covered by stands of red oak

but varied micro-relief promotes some growth of sugar maple and white birch in more moist situations. Thin soils on high ground are dominated by juniper shrubs that may be associated with small oaks. Small bare rocky sites are scattered intermittently throughout the juniper shrub areas.

B. Unique Associations

No rare or endangered species or unique associations were observed in the upland biotope on the Little Silver Lake property.

C. Species of Economic Importance

During the site reconnaissance on November 16, 1978, three ruffed grouse were flushed from juniper shrubs in the upland areas. These birds are an important upland game species that are hunted during the autumn months. Another game species, snowshoe hare, inhabit areas of scrub vegetation and secondary growth as well. Although the property has moderately severe limitations to the production of ungulates (Canada Land Inventory 1970), a deer was observed on the site in November. Deer may inhabit or wander through the property where there is suitable browse and cover.

The land has severe limitations to the growth of commercial forests of red pine and red oak because of either soil moisture excesses or thin soil layers (Canada Land Inventory 1971).

D. Constraints to Development

Upland clearings have a low degree of ecological sensitivity and are suitable for development.

5.2 Lowland Biotope (corresponds to parts of Terrain Unit 3, 4, 5)

A. Description and Distribution

The lowland biotope includes the low-lying, well-drained areas where there are deeper soil deposits and also areas associated with the swamps. The forest stand is composed largely of poplars and sugar maples with white birch and some eastern white cedar. The understory consists of red osier dogwood, willows and ash shrubs. There is a stand of white pine along ridges and low-lying areas between the north end of the small lake and Little Silver Lake. The stand composition changes to a predominance of oak on the ridge hillsides as the soil moisture conditions become drier. Oak stands are not mature but consist of scattered mature individuals among younger trees.

B. Unique Associations

No rare or endangered species or unique associations were observed in the lowland biotope.

C. Species of Economic Importance

The low-lying areas of the Little Silver Lake property have severe limitations to the growth of hard maple commercial forests because of moisture excesses and shallow soil conditions. Ruffed grouse and snowshoe hare are found in virtually all areas of the acreage including the lowland biotope.

D. Constraints to Development

The tree growth in the low areas prevents surficial erosion and is an important input of organic matter (via leaf litter) into the soil. Existing vegetation on the hillsides helps to stabilize the thin soil that has been deposited on these slopes. Tree cutting

should be minimized therefore, during construction activities in this terrain unit.

5.3 Open Field Biotope (corresponds to parts of Terrain Units 3 & 4)

A. Description and Distribution

The rugged and shallow and stony soils place severe limitations on agricultural practices in these terrain units. While some open field areas were once cleared for agriculture, they are currently either being used for grazing purposes while other clearings have been left fallow for several years. Unused fields have early successional growths of golden rod, milkweed, staghorn sumac, hawthorn and some poplar saplings. Areas with scrub vegetation provide habitat for ruffed grouse, eastern cottontails, snowshoe hare, raccoon and fox.

B. Unique Associations

No rare or endangered species or unique associations are present in the open field biotope.

C. Species of Economic Importance

Upland game associated with fields, clearings and the vegetation on the edge of these openings include ruffed grouse and snowshoe hare. As previously stated, deer may wander through the property where there is suitable browse and cover.

D. Constraints to Development

The fields and clearings are the most suitable areas for development. These sites, some originally chosen for use as pastures, are the best drained and deepest soiled areas on the property.

They do not have a high degree of ecological sensitivity as they have been disturbed by human activity in the recent past.

5.4 Marsh and Swamp Biotope (corresponds to Terrain Unit 6)

A. Description and Distribution

At the south end of the small lake, along the shallow margins and extending to the Westport-Maberly Road, marsh vegetation consisting of cattails, bulrushes and grasses grow in submerged and water-logged soils. Ash, dogwood and willow shrubs are proliferant in the poorly drained conditions that exist around the perimeter of the lake.

Throughout the rest of the property, there are extensive permanently flooded low-lying areas. These swamps, created by beavers disturbing the natural drainage, are filled with dead and rotting trees, notably poplar. Shrubs, including willow and ash, grow on wet sites at the swamp edges.

B. Unique Associations

The presence of wetlands in a relatively undisturbed tract of land is conducive to a diverse group of wildlife. There is evidence of beaver activity at all the swamp sites and muskrats are almost always associated with them. Although this land is classed as having severe limitations to the production of waterfowl according to Land Capability for Wildlife - Waterfowl, Canada Land Inventory 1971, the extensive swampy sites and the marsh area of the small lake serve as important resting and feeding locations for migrants. They may also support a small resident breeding population for some species of ducks.

Marshes and swamps are also excellent habitats and important production centres for aquatic invertebrates, amphibians and reptiles.

C. Species of Economic Importance

Waterfowl such as mallards, black ducks and blue-winged teal are important game species despite the severe limitations to waterfowl production classification by the Canada Land Inventory 1971. Beaver and muskrat are fur-bearers that inhabit most of the existing wetland areas but their economic potential is unknown.

D. Constraints to Development

Marshes and swamps are vulnerable to pollution by increased inputs of natural and unnatural substances from development. Road and building construction near marshes and swamps may cause some siltation, particularly in the shallow waters. Inputs of nutrients from sewage effluents change the chemical conditions of the water. Eutrophication destroys the floating and emergent vegetation and is extremely detrimental to populations of waterfowl and other wetland wildlife. No development activities such as dredging or infilling should be permitted in this terrain unit.

5.5 Lake and Shoreline Biotope (Mapped on Figure 2)

A. Description and Distribution

Included in the property is approximately 4.0 km of Little Silver Lake shoreline and 1.5 km of shoreline of the small lake. There is little emergent aquatic vegetation on Little Silver Lake as shore is rocky, steep-sloped and in most locations forested. The depth of water increases rapidly from the lake edge. This lake is a

warm water fishery with such species as smallmouth bass and yellow perch. The small, shallow lake has a rocky shoreline except at the south end where emergent aquatic vegetation is proliferant. Yellow perch and introduced rainbow trout inhabit the lake at the present time. Beaver activity was observed and the lake probably serves as an important resting and feeding site for some migrants and may support a small resident duck population.

B. Unique Associations

No rare or endangered species or unique associations were observed in the lake and shoreline biotope.

C. Species of Economic Importance

Surface-feeding ducks such as mallards, blacks and blue-winged teal as well as diving ducks like ring-necked ducks, scaup, goldeneye and bufflehead are common game species of waterfowl. Sport fishes from a warm water fishery like Little Silver Lake include large and/or smallmouth bass, yellow perch, walleye and northern pike. Approximately 2,000 rainbow trout have been planted in the small lake. Successful over wintering of the trout will not be known until the spring of 1979, and breeding is unlikely.

D. Constraints to Development

As settling basins, the lakes are sensitive to inputs of sewage and silt. Little Silver Lake and the adjacent small lake are relatively small and not tolerant to inputs of effluents from residential developments. In comparison, other much larger lakes are not eutrophied because of unnatural nutrient enrichment from cottage disposal systems.

The fisheries may be affected as a result of damage to spawning areas. The trout in the small lake will tend to move upstream (in this case into Little Silver Lake) if the conditions become too severe.

Accordingly, we endorse the development recommendations made for these lakes by the Ministry of Natural Resources (Little Silver Lake Study Report, M.N.R., Lanark District, December 1978);

1. All development, including septic tanks and tilefields should be set back at least 100 feet from the highwater mark. If the physical limitations of a particular lot indicate a greater setback is required, the Ministry will recommend this when reviewing the specific proposal.

2. The disturbance of the natural vegetation within 100 feet of the highwater mark should be discouraged. This will help to stabilize soils, hold back nutrients, and protect the scenic quality of the shoreline.

3. No development, including dredging and/or filling should be permitted within the wetland areas shown on the accompanying map.

4. Future development should be compatible with existing uses on the lake, and should be consistent with the lake's ability to support the proposed area.

Respectfully submitted

A handwritten signature in cursive script that reads "Derek P. Smith".

Derek P. Smith M.Sc. FGAC

6.0 Selected References

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B. Section 5

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APPENDIX A

Grain Size Analysis

Matrix of

Glacial Till Ground Moraine

GRAIN SIZE DISTRIBUTION

JOHN D. PATERSON & ASSOCIATES LTD.

Consulting Engineers and Geologists

1479 LAPERRIERE AVE.

OTTAWA, CANADA K1Z 7S8

SOIL SAMPLE DESCRIPTION:

Sand - Gravel.

LOCATION:

PROJECT: Water & Earth Science Associates

BORE HOLE NO.

SAMPLE NO. 4

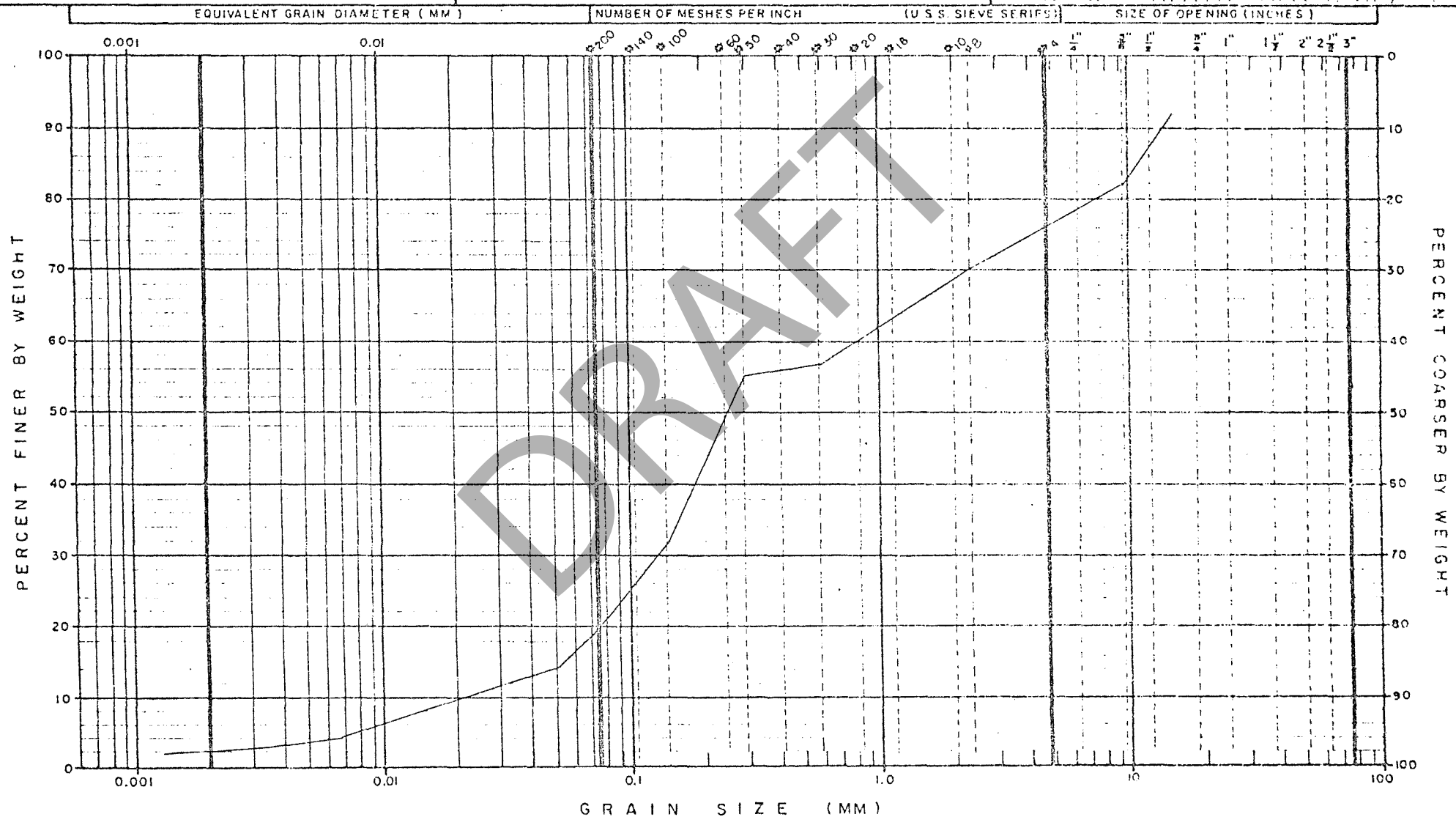
LAB. NO.

TESTED BY: S.W.

DATE:

CHECKED BY: S.W.

DATE: Jan, 15/79



REMARKS:

EFFECTIVE GRAIN SIZE, D_{10} (CM)

D_{60} (CM)

UNIFORMITY COEFFICIENT, C_u =

APPENDIX B

Water Well Logs

Concessions 7, 8, 9

Lots 10 - 16

South Sherbrooke Township

CON	LOT	UTM EASTING NORTHING	ELEV FEET	CSG DIA INS	KIND OF WATER	WATER FOUND FEET	STAT LVL FEET	PUMP LVL FEET	TEST RATE GPM	TEST TIME HR/MN	WATER USE	OWNER/LOG
7	10	377220 4963650	600	6	FR	58	11	76	5	1/00	DO	VILLENEUVE F Tps1 Msnd 0005 Shle 0015 Grey Grnt 0076
7	15	379350 4965130	575	2	FR	78	10	50	1	2/00	DO	SMITH L Msnd 0014 Red Grnt 0115
7	16	380160 4965295	585	6	FR	40	16	48	4	2/00	DO	CONROY J Brwn Tps1 0001 Whit Lmsn 0036 Blck Grnt 0048
8	11	377220 4964780	610	6	FR	52	10	25	2	1/00	DO	MUNRO S Tps1 0001 Fill Bldr 0012 Red Grnt 0062
8	13	378040 4965430	609	6	FR	32 64	8	65	2	1/00	ST DO	BRIGGS A Msnd 0007 Blck Grnt 0065
8	14	378100 4965640	600	6	FR	40	10	45	5	3/30	DO	FLEMING Cecil Brwn Msnd 0007 Blck Grnt 0050
8	14	378140 4965800	575	6	FR	35 55	20	63	1	3/00	DO	FLEMING V Brwn Tps1 0004 Rock 0018 Blck Grnt 0063
8	14	378300 4965870	565	6	FR	40	25	45	5	/30	DO	MARSHALL H Fill 0012 Shle 0016 Grnt 0054
8	14	378500 4965620	625	6	FR	27	11	15	45	/30	DO	MCFARLAND CONSTRUCT Msnd 0004 Red Grnt 0033

CON	LOT	UTM EASTING NORTHING	ELEV FEET	CSG DIA INS	KIND OF WATER	WATER FOUND FEET	STAT LVL FEET	PUMP LVL FEET	TEST RATE GPM	TEST TIME HR/MN	WATER USE	OWNER/LOG
9	11	376550 4965345	585	6	FR	50	15	63	1	1/00	DO	GRAY A Msnd 0004 Blck Grnt 0063
9	13	377400 4965620	590	6	FR	68	4	70	1	3/15	DO	MACDONNEL B Tps1 Msnd 0008 Grey Grnt 0068 Grn Grnt Shle 0069 Blck Grnt 0070
9	13	377450 4966277	650	6	FR	115	22	126	5	1/00	ST DO	CONBOY R Shle 0003 Blck Grnt 0126
9	14	377615 4966220	650	6	FR	40	12	16	30	1/00	PS	MABERLY SCHOOL Msnd 0001 Grey Grnt 0048
9	14	377670 4966690	550	6	FR	80	18	100	7	1/30	DO	VANALSTINE K Brwn Tps1 0001 Grey Grnt 0018 Red Grnt 0040 Grey Grnt 0100
9	14	378020 4965820	595	6	FR	35 80	10	75	2	1/00	DO	ORSER W Clay 0001 Bldr 0011 Grnt 0085
9	16	378400 4967791	607	6	FR	65	33	72	4	1/30	ST	VANALSTINE D Whit Lmsn 0072
9	16	378740 4967676	620	7	FR	30	18	56	5	1/30	DO	VANALSTINE D Brwn Tps1 0015 Grv1 0018 Whit Lmsn 0056

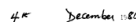
APPENDIX B

Topographic Survey (1980)

DRAFT



ADDENDUM NO. 2

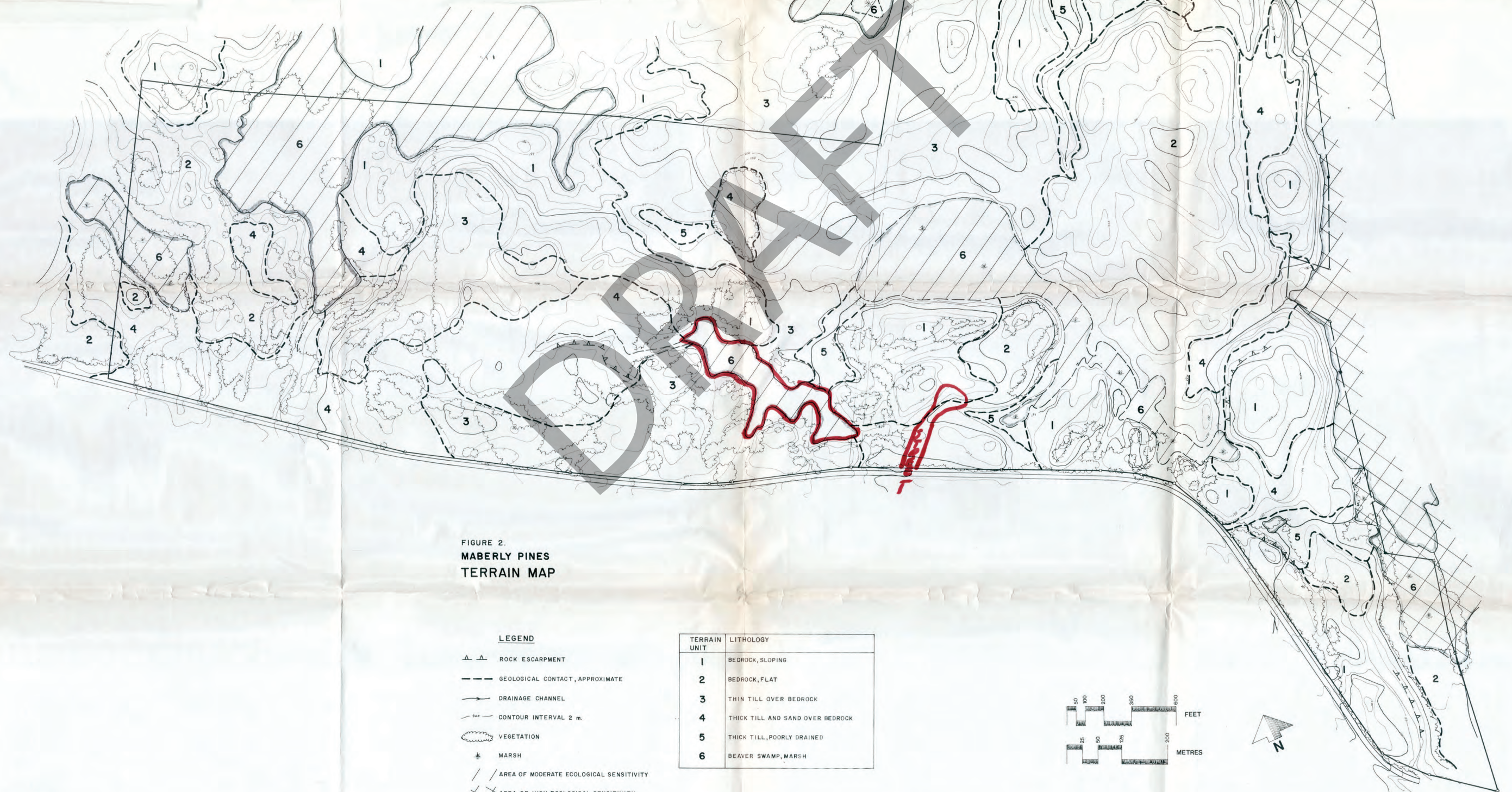
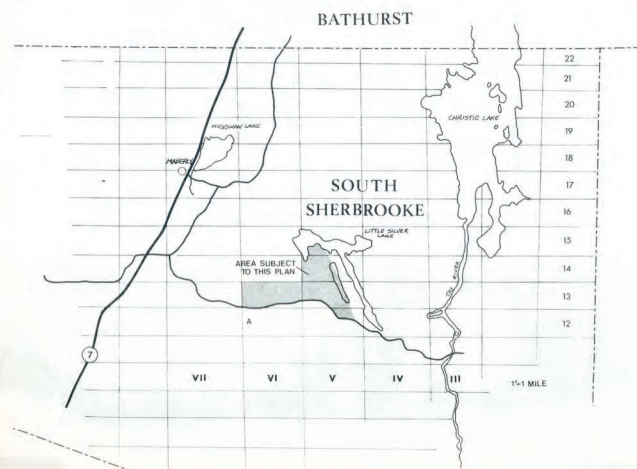


APPENDIX C

Terrain Analysis Map (WESA, 1979)

DRAFT





APPENDIX D

MECP Water Well Records

DRAFT





Ontario

MINISTRY OF THE ENVIRONMENT
The Ontario Water Resources Act

WATER WELL RECORD

31 4/5 E

1. PRINT ONLY IN SPACES PROVIDED
2. CHECK ☒ CORRECT BOX WHERE APPLICABLE

11

13503579

35014

CON

07

COUNTY OR DISTRICT LANARK	TOWNSHIP, BOROUGH, CITY, TOWN, VILLAGE SOUTH SHERBROOKE	CON., BLOCK, TRACT, SURVEY, ETC. 7	LOT 013
MABERLY, R.R.# 3,			DATE COMPLETED DAY 10 MO. 09 YR. 73
10-13 9.6	14-17 4.4	18-21 5.8	22-25 4
26-29 0.4	30-33 3.2	34-37 5	38-41 2.6

LOG OF OVERBURDEN AND BEDROCK MATERIALS (SEE INSTRUCTIONS)

GENERAL COLOUR	MOST COMMON MATERIAL	OTHER MATERIALS	GENERAL DESCRIPTION	DEPTH - FEET	
				FROM	TO
BLACK	EARTH			0	8
BLACK	GRANITE		HARD	8	50
RED	GRANITE		HARD	50	80

31	0008801	0050821	0080721
32			

41 WATER RECORD	
WATER FOUND AT - FEET	KIND OF WATER
10-13 0074	1 <input checked="" type="checkbox"/> FRESH 3 <input type="checkbox"/> SULPHUR 2 <input type="checkbox"/> SALTY 4 <input type="checkbox"/> MINERAL
15-18	1 <input type="checkbox"/> FRESH 3 <input type="checkbox"/> SULPHUR 2 <input type="checkbox"/> SALTY 4 <input type="checkbox"/> MINERAL
20-23	1 <input type="checkbox"/> FRESH 3 <input type="checkbox"/> SULPHUR 2 <input type="checkbox"/> SALTY 4 <input type="checkbox"/> MINERAL
25-28	1 <input type="checkbox"/> FRESH 3 <input type="checkbox"/> SULPHUR 2 <input type="checkbox"/> SALTY 4 <input type="checkbox"/> MINERAL
30-33	1 <input type="checkbox"/> FRESH 3 <input type="checkbox"/> SULPHUR 2 <input type="checkbox"/> SALTY 4 <input type="checkbox"/> MINERAL

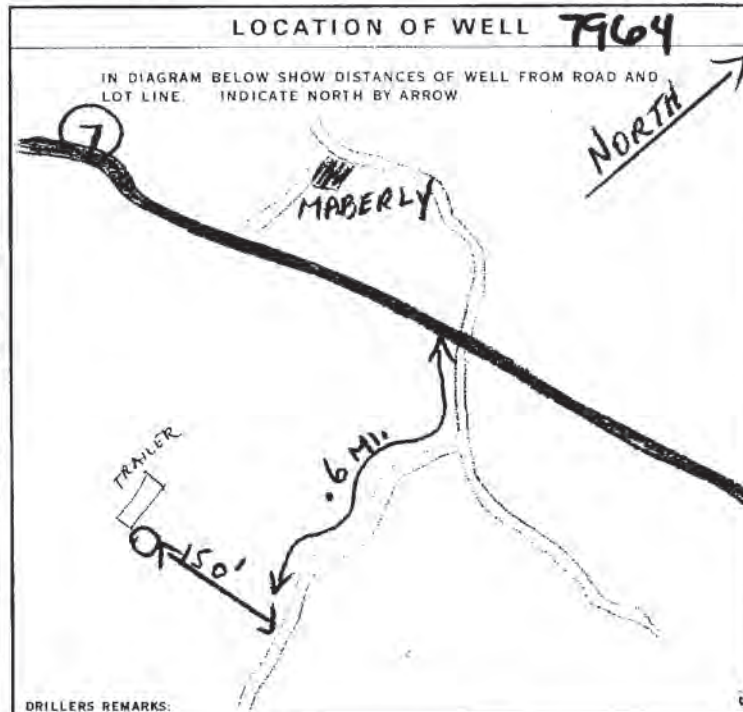
51 CASING & OPEN HOLE RECORD			
INSIDE DIAM. INCHES	MATERIAL	WALL THICKNESS INCHES	DEPTH - FEET
10-11 06	1 <input checked="" type="checkbox"/> STEEL 2 <input type="checkbox"/> GALVANIZED 3 <input type="checkbox"/> CONCRETE 4 <input type="checkbox"/> OPEN HOLE	.188	FROM 0 TO 0022
17-18 06	1 <input type="checkbox"/> STEEL 2 <input type="checkbox"/> GALVANIZED 3 <input type="checkbox"/> CONCRETE 4 <input type="checkbox"/> OPEN HOLE		FROM 22 TO 0080
24-25	1 <input type="checkbox"/> STEEL 2 <input type="checkbox"/> GALVANIZED 3 <input type="checkbox"/> CONCRETE 4 <input type="checkbox"/> OPEN HOLE		FROM 27 TO 30

SCREEN	SIZE (S) OF OPENING (SLOT NO.)	DIAMETER	LENGTH
		INCHES	FEET
	MATERIAL AND TYPE	DEPTH TO TOP OF SCREEN	FEET

61 PLUGGING & SEALING RECORD	
DEPTH SET AT - FEET	MATERIAL AND TYPE (CEMENT GROUT, LEAD PACKER, ETC.)
FROM TO	
10-13 14-17	
18-21 22-25	
26-29 30-33 80	

71 PUMPING TEST METHOD	10 PUMPING RATE	11-14 DURATION OF PUMPING
1 <input checked="" type="checkbox"/> PUMP 2 <input type="checkbox"/> BAILER	00 20	15-16 HOURS 01 17-18 MINS 45
STATIC LEVEL	WATER LEVEL END OF PUMPING	WATER LEVELS DURING
19-21 018	22-24 075	15 MINUTES 26-28 018 30 MINUTES 29-31 018 45 MINUTES 32-34 018 60 MINUTES 35-37 018
IF FLOWING GIVE RATE	PUMP INTAKE SET AT	WATER AT END OF TEST
	70	1 <input checked="" type="checkbox"/> CLEAR 2 <input type="checkbox"/> CLOUDY
RECOMMENDED PUMP TYPE	RECOMMENDED PUMP SETTING	RECOMMENDED PUMPING RATE
<input type="checkbox"/> SHALLOW <input checked="" type="checkbox"/> DEEP	070	0015
50-53	000.4 GPM./FT. SPECIFIC CAPACITY	

54 FINAL STATUS OF WELL	1 <input checked="" type="checkbox"/> WATER SUPPLY 2 <input type="checkbox"/> OBSERVATION WELL 3 <input type="checkbox"/> TEST HOLE 4 <input type="checkbox"/> RECHARGE WELL	5 <input type="checkbox"/> ABANDONED, INSUFFICIENT SUPPLY 6 <input type="checkbox"/> ABANDONED, POOR QUALITY 7 <input type="checkbox"/> UNFINISHED
55-56 WATER USE	1 <input checked="" type="checkbox"/> DOMESTIC 2 <input type="checkbox"/> STOCK 3 <input type="checkbox"/> IRRIGATION 4 <input type="checkbox"/> INDUSTRIAL 5 <input type="checkbox"/> OTHER	5 <input type="checkbox"/> COMMERCIAL 6 <input type="checkbox"/> MUNICIPAL 7 <input type="checkbox"/> PUBLIC SUPPLY 8 <input type="checkbox"/> COOLING OR AIR CONDITIONING 9 <input type="checkbox"/> NOT USED
57 METHOD OF DRILLING	1 <input type="checkbox"/> CABLE TOOL 2 <input type="checkbox"/> ROTARY (CONVENTIONAL) 3 <input type="checkbox"/> ROTARY (REVERSE) 4 <input type="checkbox"/> ROTARY (AIR) 5 <input checked="" type="checkbox"/> AIR PERCUSSION	6 <input type="checkbox"/> BORING 7 <input type="checkbox"/> DIAMOND 8 <input type="checkbox"/> JETTING 9 <input type="checkbox"/> DRIVING



NAME OF WELL CONTRACTOR J.R. THOMPSON	LICENCE NUMBER 4905
ADDRESS R.R.# 1, WESTPORT, ONTARIO	
NAME OF DRILLER OR BORER DONALD SMITH	LICENCE NUMBER
SIGNATURE OF CONTRACTOR <i>J.R. Thompson</i>	SUBMISSION DATE DAY 30 MO. 1 YR. 74

DATA SOURCE 1	CONTRACTOR 4905	DATE RECEIVED 120274
DATE OF INSPECTION 19 Jun 75	INSPECTOR K.P/R. Doughty	
REMARKS:		P WI



Ministry
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Ontario

The Ontario Water Resources Act

31 C15E

WATER WELL RECORD

1. PRINT ONLY IN SPACES PROVIDED
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11

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MUNICIP

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07

COUNTY OR DISTRICT	TOWNSHIP, BOROUGH, CITY, TOWN, VILLAGE	CON. BLOCK, TRACT, SURVEY, ETC.	LOT
1	SOUTH SHELBROOKE	7	014
DATE COMPLETED			48-53
R#1 MABERLY ONT. RCH 280			DAY 08 MO 06 YR 81
2	364399	5	0650
3	6	26	

LOG OF OVERBURDEN AND BEDROCK MATERIALS (SEE INSTRUCTIONS)

GENERAL COLOUR	MOST COMMON MATERIAL	OTHER MATERIALS	GENERAL DESCRIPTION	DEPTH - FEET	
				FROM	TO
	SAND			0	18
	GRANITE			18	150

31	0018 28	0150 21
32		

41 WATER RECORD	
WATER FOUND AT - FEET	KIND OF WATER
10-13 0142	1 <input checked="" type="checkbox"/> FRESH 3 <input type="checkbox"/> SULPHUR 2 <input type="checkbox"/> SALTY 4 <input type="checkbox"/> MINERAL
15-18	1 <input type="checkbox"/> FRESH 3 <input type="checkbox"/> SULPHUR 2 <input type="checkbox"/> SALTY 4 <input type="checkbox"/> MINERAL
20-23	1 <input type="checkbox"/> FRESH 3 <input type="checkbox"/> SULPHUR 2 <input type="checkbox"/> SALTY 4 <input type="checkbox"/> MINERAL
25-28	1 <input type="checkbox"/> FRESH 3 <input type="checkbox"/> SULPHUR 2 <input type="checkbox"/> SALTY 4 <input type="checkbox"/> MINERAL
30-33	1 <input type="checkbox"/> FRESH 3 <input type="checkbox"/> SULPHUR 2 <input type="checkbox"/> SALTY 4 <input type="checkbox"/> MINERAL

51 CASING & OPEN HOLE RECORD			
INSIDE DIAM. INCHES	MATERIAL	WALL THICKNESS INCHES	DEPTH - FEET
10-11 06	1 <input checked="" type="checkbox"/> STEEL		13-16
12-13 06	2 <input type="checkbox"/> GALVANIZED		17-18
14-15 06	3 <input type="checkbox"/> CONCRETE		20-23
16-17 06	4 <input type="checkbox"/> OPEN HOLE		24-25
18-19 06	5 <input type="checkbox"/> GALVANIZED		26-27
20-21 06	6 <input type="checkbox"/> CONCRETE		28-29
22-23 06	7 <input type="checkbox"/> OPEN HOLE		30-33

SCREEN	SIZE OF OPENING - SLOT NO. 1	37-38	DIAMETER	39-40	LENGTH
	MATERIAL AND TYPE		INCHES	FEET	
			DEPTH TO TOP OF SCREEN	41-44	45

61 PLUGGING & SEALING RECORD		
DEPTH SET AT - FEET	MATERIAL AND TYPE	TEST GROUT LEAD PACKER ETC.
10-13	14-17	
18-21	22-25	
26-29	30-33	80

71 PUMPING TEST METHOD		10 PUMPING RATE	11-14 DURATION OF PUMPING
1 <input checked="" type="checkbox"/> PUMP 2 <input type="checkbox"/> BAILER		0002	15-16 HOURS 17-18 MINS
STATIC LEVEL	WATER LEVEL END OF PUMPING	WATER LEVELS DURING	
19-21 009	22-24 149	15 MINUTES 26-28 123	30 MINUTES 29-31 097
		45 MINUTES 32-34 078	60 MINUTES 35-37 060
IF FLOWING GIVE RATE	PUMP INTAKE SET AT	WATER AT END OF TEST	
	145	42	
RECOMMENDED PUMP TYPE	RECOMMENDED PUMP SETTING	RECOMMENDED PUMPING RATE	
<input type="checkbox"/> SHALLOW <input checked="" type="checkbox"/> DEEP	145	0002	

LOCATION OF WELL	
IN DIAGRAM BELOW SHOW DISTANCES OF WELL FROM ROAD AND LOT LINE INDICATE NORTH BY ARROW	
DRILLERS REMARKS	

FINAL STATUS OF WELL	1 <input checked="" type="checkbox"/> WATER SUPPLY 5 <input type="checkbox"/> ABANDONED - INSUFFICIENT SUPPLY 2 <input type="checkbox"/> OBSERVATION WELL 6 <input type="checkbox"/> ABANDONED - POOR QUALITY 3 <input type="checkbox"/> TEST HOLE 7 <input type="checkbox"/> UNFINISHED 4 <input type="checkbox"/> RECHARGE WELL
WATER USE	1 <input checked="" type="checkbox"/> DOMESTIC 5 <input type="checkbox"/> COMMERCIAL 2 <input type="checkbox"/> STOCK 6 <input type="checkbox"/> MUNICIPAL 3 <input type="checkbox"/> IRRIGATION 7 <input type="checkbox"/> PUBLIC SUPPLY 4 <input type="checkbox"/> INDUSTRIAL 8 <input type="checkbox"/> COOLING OR AIR CONDITIONING 9 <input type="checkbox"/> NOT USED
METHOD OF DRILLING	1 <input type="checkbox"/> CABLE TOOL 6 <input type="checkbox"/> BORING 2 <input type="checkbox"/> ROTARY (CONVENTIONAL) 7 <input type="checkbox"/> DIAMOND 3 <input type="checkbox"/> ROTARY (REVERSE) 8 <input type="checkbox"/> JETTING 4 <input type="checkbox"/> ROTARY (AIR) 9 <input type="checkbox"/> DRIVING 5 <input checked="" type="checkbox"/> AIR PERCUSSION

CONTRACTOR	NAME OF WELL CONTRACTOR	LICENCE NUMBER
	J.R. THOMPSON	4905
	ADDRESS	
	RR#1 WESTPORT	
CONTRACTOR	NAME OF DRILLER OR BORER	LICENCE NUMBER
	DONALD SMITH	
	SIGNATURE OF CONTRACTOR	SUBMISSION DATE
	J.R. Thompson	DAY 15 NO 2 YR 81

OFFICE USE ONLY	DATE SOURCE	CONTRACTOR	DATE RECEIVED
	1	4905	14 04 82
	DATE OF INSPECTION	INSPECTOR	
	REMARKS		



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WATER WELL RECORD

31C15E

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MUNICIP

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CDN

05

COUNTY OR DISTRICT

London

TOWNSHIP, BOROUGH, CITY, TOWN, VILLAGE

South Sherbrooke

CON BLOCK TRACT SURVEY ETC

5

LOT

05

DATE COMPLETED

DAY 28 MO 06 YR 83

61599

5

0650

6

28

LOG OF OVERBURDEN AND BEDROCK MATERIALS (SEE INSTRUCTIONS)

GENERAL COLOUR	MOST COMMON MATERIAL	OTHER MATERIALS	GENERAL DESCRIPTION	DEPTH - FEET	
				FROM	TO
Red	sand			0	9
Gray	Limestone			9	60
Brown & Gray	Limestone			60	80
Gray	Limestone			80	225



31 0009728 0060215 0080615 0225215

32

41 WATER RECORD

WATER FOUND FEET	KIND OF WATER
0-15	1 <input checked="" type="checkbox"/> FRESH 3 <input type="checkbox"/> SULPHUR 4 <input type="checkbox"/> MINERAL
15-18	1 <input type="checkbox"/> FRESH 3 <input type="checkbox"/> SULPHUR 4 <input type="checkbox"/> MINERAL
20-23	1 <input type="checkbox"/> FRESH 3 <input type="checkbox"/> SULPHUR 4 <input type="checkbox"/> MINERAL
25-28	1 <input type="checkbox"/> FRESH 3 <input type="checkbox"/> SULPHUR 4 <input type="checkbox"/> MINERAL
30-33	1 <input type="checkbox"/> FRESH 3 <input type="checkbox"/> SULPHUR 4 <input type="checkbox"/> MINERAL

51 CASING & OPEN HOLE RECORD

INSIDE DIAM INCHES	MATERIAL	WALL THICKNESS INCHES	DEPTH - FEET
06	STEEL	188	0-188
12-18	STEEL		188-20-23
24-25	STEEL		20-23-27-30

61 PLUGGING & SEALING RECORD

DEPTH SET AT - FEET	MATERIAL AND TYPE	CEMENT GROUT LEAD PACKER ETC
10-13		
18-21		
26-29		

71 PUMPING TEST

PUMPING TEST METHOD	PUMPING RATE	DURATION OF PUMPING
1 <input checked="" type="checkbox"/> PUMP 2 <input type="checkbox"/> SAILER	#2 0002	01 15-16 00 17-18
STATIC LEVEL	WATER LEVEL END OF PUMPING	WATER LEVELS DURING
016	225	15 MINUTES 26-28 30 MINUTES 29-31 45 MINUTES 32-34 60 MINUTES 35-37
IF FLOWING GIVE RATE	PUMP INTAKE SET AT	WATER AT END OF TEST
	225	1 <input checked="" type="checkbox"/> CLEAR 2 <input type="checkbox"/> CLOUDY
RECOMMENDED PUMP TYPE	RECOMMENDED PUMP SETTING	RECOMMENDED PUMPING RATE
<input type="checkbox"/> SHALLOW <input checked="" type="checkbox"/> DEEP	200	0002

LOCATION OF WELL

IN DIAGRAM BELOW SHOW DISTANCES OF WELL FROM ROAD AND LOT LINE INDICATE NORTH BY ARROW

FINAL STATUS OF WELL

1 <input checked="" type="checkbox"/> WATER SUPPLY	5 <input type="checkbox"/> ABANDONED, INSUFFICIENT SUPPLY
2 <input type="checkbox"/> OBSERVATION WELL	6 <input type="checkbox"/> ABANDONED, POOR QUALITY
3 <input type="checkbox"/> TEST HOLE	7 <input type="checkbox"/> UNFINISHED
4 <input type="checkbox"/> RECHARGE WELL	

WATER USE

1 <input checked="" type="checkbox"/> DOMESTIC	5 <input type="checkbox"/> COMMERCIAL
2 <input type="checkbox"/> STOCK	6 <input type="checkbox"/> MUNICIPAL
3 <input type="checkbox"/> IRRIGATION	7 <input type="checkbox"/> PUBLIC SUPPLY
4 <input type="checkbox"/> INDUSTRIAL	8 <input type="checkbox"/> COOLING OR AIR CONDITIONING
	9 <input type="checkbox"/> NOT USED

METHOD OF DRILLING

1 <input type="checkbox"/> CABLE TOOL	6 <input type="checkbox"/> BORING
2 <input type="checkbox"/> ROTARY (CONVENTIONAL)	7 <input type="checkbox"/> DIAMOND
3 <input type="checkbox"/> ROTARY (REVERSE)	8 <input type="checkbox"/> JETTING
4 <input checked="" type="checkbox"/> ROTARY (AIR)	9 <input type="checkbox"/> DRIVING
5 <input type="checkbox"/> AIR PERCUSSION	

CONTRACTOR

NAME OF WELL CONTRACTOR	LICENCE NUMBER
Shif Hall Ltd	2558
ADDRESS	
Mc Donalds Corner	
NAME OF DRILLER OR BORER	LICENCE NUMBER
Shif Hall	2558
SIGNATURE OF CONTRACTOR	SUBMISSION DATE
Shif Hall	DAY 29 MO 7 YR 83

OFFICE USE ONLY

DATA SOURCE	CONTRACTOR	DATE
1	2558	12 03 84
DATE OF INSPECTION	INSPECTOR	
REMARKS		

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Ontario

The Ontario Water Resources Act

WATER WELL RECORD

31C15E

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3506756

MUNICIPALITY 35014

CON 013

05

COUNTY OR DISTRICT

Lanark

TOWNSHIP BOROUGH CITY TOWN VILLAGE

South Sherbrooke

CON BLOCK TRACT SURVEY ETC.

5

LOT

013

DATE COMPLETED

DAY 27 MO 06 YR 83

61599

5

0650

6

26

LOG OF OVERBURDEN AND BEDROCK MATERIALS (SEE INSTRUCTIONS)

GENERAL COLOUR	MOST COMMON MATERIAL	OTHER MATERIALS	GENERAL DESCRIPTION	DEPTH FEET	
				FROM	TO
red & sand & boulders				0'	3'
gray limestone				3'	64'

MOE
WF-17

31 000372813 0064215

32

41 WATER RECORD

WATER FOUND AT - FEET	KIND OF WATER
0060	1 <input checked="" type="checkbox"/> FRESH 3 <input type="checkbox"/> SULPHUR 2 <input type="checkbox"/> SALTY 4 <input type="checkbox"/> MINERAL
15-18	1 <input type="checkbox"/> FRESH 3 <input type="checkbox"/> SULPHUR 2 <input type="checkbox"/> SALTY 4 <input type="checkbox"/> MINERAL
20-23	1 <input type="checkbox"/> FRESH 3 <input type="checkbox"/> SULPHUR 2 <input type="checkbox"/> SALTY 4 <input type="checkbox"/> MINERAL
25-28	1 <input type="checkbox"/> FRESH 3 <input type="checkbox"/> SULPHUR 2 <input type="checkbox"/> SALTY 4 <input type="checkbox"/> MINERAL
30-33	1 <input type="checkbox"/> FRESH 3 <input type="checkbox"/> SULPHUR 2 <input type="checkbox"/> SALTY 4 <input type="checkbox"/> MINERAL

51 CASING & OPEN HOLE RECORD

INSIDE DIAM INCHES	MATERIAL	WELL THICKNESS INCHES	DEPTH FEET
06	1 <input checked="" type="checkbox"/> STEEL 2 <input type="checkbox"/> GALVANIZED 3 <input type="checkbox"/> CONCRETE 4 <input type="checkbox"/> OPEN HOLE	188	0' 0022'
17-18	1 <input type="checkbox"/> STEEL 2 <input type="checkbox"/> GALVANIZED 3 <input type="checkbox"/> CONCRETE 4 <input type="checkbox"/> OPEN HOLE		20-23
24-25	1 <input type="checkbox"/> STEEL 2 <input type="checkbox"/> GALVANIZED 3 <input type="checkbox"/> CONCRETE 4 <input type="checkbox"/> OPEN HOLE		27-30

61 PLUGGING & SEALING RECORD

DEPTH SET AT - FEET	MATERIAL AND TYPE	CEMENT GROUT LEAD PACKER ETC.
10-13	14-17	
18-21	22-25	
26-29	30-33	80

71 PUMPING TEST

PUMPING TEST METHOD: 1 ☒ PUMP 2 ☐ BAILER

PUMPING RATE: 0004 GPM

DURATION OF PUMPING: 01 00 HOURS

STATIC LEVEL: 018 FEET

WATER LEVEL END OF PUMPING: 22-24 FEET

WATER LEVELS DURING PUMPING:

15 MINUTES	30 MINUTES	45 MINUTES	60 MINUTES
018	018	018	018

IF FLOWING, GIVE RATE: 64 GPM

PUMP INTAKE SET AT: 054 FEET

WATER AT END OF TEST: 0004 GPM

RECOMMENDED PUMP TYPE: 1 ☐ SHALLOW 2 ☒ DEEP

RECOMMENDED PUMP SETTING: 054 FEET

RECOMMENDED PUMPING RATE: 0004 GPM

LOCATION OF WELL

IN DIAGRAM BELOW SHOW DISTANCES OF WELL FROM ROAD AND LOT LINE. INDICATE NORTH BY ARROW.

well

Rain Bow Lake

DRILLERS REMARKS

FINAL STATUS OF WELL: 1

WATER USE: 01

METHOD OF DRILLING: 4

1 ☒ WATER SUPPLY
2 ☐ OBSERVATION WELL
3 ☐ TEST HOLE
4 ☐ RECHARGE WELL
5 ☐ ABANDONED, INSUFFICIENT SUPPLY
6 ☐ ABANDONED, POOR QUALITY
7 ☐ UNFINISHED

1 ☒ DOMESTIC
2 ☐ STOCK
3 ☐ IRRIGATION
4 ☐ INDUSTRIAL
5 ☐ COMMERCIAL
6 ☐ MUNICIPAL
7 ☐ PUBLIC SUPPLY
8 ☐ COOLING OR AIR CONDITIONING
9 ☐ NOT USED

1 ☐ CABLE TOOL
2 ☐ ROTARY (CONVENTIONAL)
3 ☐ ROTARY (REVERSE)
4 ☒ ROTARY (AIR)
5 ☐ AIR PERCUSSION
6 ☐ BORING
7 ☐ DIAMOND
8 ☐ JETTING
9 ☐ DRIVING

CONTRACTOR

NAME OF WELL CONTRACTOR: Dry Hall Ltd

LICENCE NUMBER: 2558

ADDRESS: McDonalds Corners Ont

NAME OF DRILLER OR BORER: Dry Hall

LICENCE NUMBER: 2558

SIGNATURE OF CONTRACTOR: Dry Hall

SUBMISSION DATE: DAY 29 MO 30 YR 83

OFFICE USE ONLY

DATA SOURCE: 1

CONTRACTOR: 2558

DATE RECEIVED: 12 03 84

DATE OF INSPECTION: INSPECTOR:

REMARKS:



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WATER WELL RECORD

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3506757

Plan Lot 9 in
R.L. 29 C.O.N.

05

COUNTY OR DISTRICT

TOWNSHIP BOROUGH CITY TOWN VILLAGE

LOW BLOCK NO. SURVEY ETC.

Lanark

South Sherbrooke

35014

014

3-2625 Reginald Ottawa

DATE COMPLETED

DAY 24 MO 06 YR 83

62599

5

ELEVATION
0650

6

26

LOG OF OVERBURDEN AND BEDROCK MATERIALS (SEE INSTRUCTIONS)

GENERAL COLOUR	MOST COMMON MATERIAL	OTHER MATERIALS	GENERAL DESCRIPTION	DEPTH FEET	
				FROM	TO
red	sand & stone			0'	2'
gray	granite			2'	30'
brown	limestone			30'	31'
gray	limestone			31'	100'
white	limestone			100'	125'
gray	limestone			125'	170'
white	limestone			170'	180'
gray	limestone			180'	225'



31	000212812	0030221	0031615	0100215	0125115	0170215	1
32	0180115	0225215					

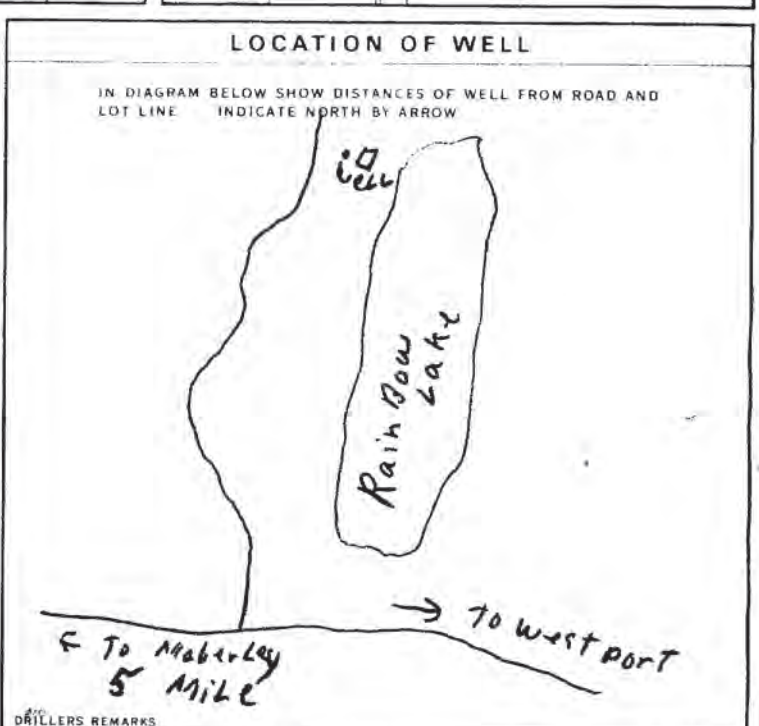
WATER RECORD	
WATER FOUND AT - FEET	KIND OF WATER
0210	1 <input checked="" type="checkbox"/> FRESH 3 <input type="checkbox"/> SULPHUR 2 <input type="checkbox"/> SALTY 4 <input type="checkbox"/> MINERAL
15-18	1 <input type="checkbox"/> FRESH 3 <input type="checkbox"/> SULPHUR 2 <input type="checkbox"/> SALTY 4 <input type="checkbox"/> MINERAL
20-23	1 <input type="checkbox"/> FRESH 3 <input type="checkbox"/> SULPHUR 2 <input type="checkbox"/> SALTY 4 <input type="checkbox"/> MINERAL
25-28	1 <input type="checkbox"/> FRESH 3 <input type="checkbox"/> SULPHUR 2 <input type="checkbox"/> SALTY 4 <input type="checkbox"/> MINERAL
30-32	1 <input type="checkbox"/> FRESH 3 <input type="checkbox"/> SULPHUR 2 <input type="checkbox"/> SALTY 4 <input type="checkbox"/> MINERAL

CASING & OPEN HOLE RECORD	
INSIDE DIAM. INCHES	MATERIAL
06"	1 <input checked="" type="checkbox"/> STEEL 2 <input type="checkbox"/> GALVANIZED 3 <input type="checkbox"/> CONCRETE 4 <input type="checkbox"/> OPEN HOLE
17-18	1 <input type="checkbox"/> STEEL 2 <input type="checkbox"/> GALVANIZED 3 <input type="checkbox"/> CONCRETE 4 <input type="checkbox"/> OPEN HOLE
24-25	1 <input type="checkbox"/> STEEL 2 <input type="checkbox"/> GALVANIZED 3 <input type="checkbox"/> CONCRETE 4 <input type="checkbox"/> OPEN HOLE

SIZE(S) OF OPENING (SLOT NO.)	DIAMETER	LENGTH
	INCHES	FEET
MATERIAL AND TYPE		DEPTH TO TOP OF SCREEN
		FEET

PLUGGING & SEALING RECORD	
DEPTH SET AT - FEET	MATERIAL AND TYPE
FROM TO	CEMENT GROUT LEAD PACKER ETC.
10-12	14-17
18-21	22-25
26-29	30-33

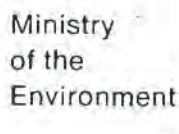
PUMPING TEST	
71	PUMPING TEST METHOD
022	1 <input checked="" type="checkbox"/> PUMP 2 <input type="checkbox"/> BAILEY
0001	PUMPING RATE
02	DURATION OF PUMPING
00	15-16 HOURS
164	17-18 MIN.
022	STATIC LEVEL
225	WATER LEVEL END OF PUMPING
225	WATER LEVELS DURING
225	15 MINUTES
225	30 MINUTES
225	45 MINUTES
225	60 MINUTES
225	IF FLOWING GIVE RATE
225	PUMP INTAKE SET AT
225	WATER AT END OF TEST
225	RECOMMENDED PUMP TYPE
225	RECOMMENDED PUMP SETTING
225	RECOMMENDED PUMPING RATE



FINAL STATUS OF WELL	
1	1 <input checked="" type="checkbox"/> WATER SUPPLY
2	2 <input type="checkbox"/> OBSERVATION WELL
3	3 <input type="checkbox"/> TEST HOLE
4	4 <input type="checkbox"/> RECHARGE WELL
5	5 <input type="checkbox"/> ABANDONED INSUFFICIENT SUPPLY
6	6 <input type="checkbox"/> ABANDONED POOR QUALITY
7	7 <input type="checkbox"/> UNFINISHED
WATER USE	
01	1 <input checked="" type="checkbox"/> DOMESTIC
2	2 <input type="checkbox"/> STOCK
3	3 <input type="checkbox"/> IRRIGATION
4	4 <input type="checkbox"/> INDUSTRIAL
5	5 <input type="checkbox"/> COMMERCIAL
6	6 <input type="checkbox"/> MUNICIPAL
7	7 <input type="checkbox"/> PUBLIC SUPPLY
8	8 <input type="checkbox"/> COOLING OR AIR CONDITIONING
9	9 <input type="checkbox"/> NOT USED
METHOD OF DRILLING	
4	1 <input type="checkbox"/> CABLE TOOL
2	2 <input type="checkbox"/> ROTARY (CONVENTIONAL)
3	3 <input type="checkbox"/> ROTARY (REVERSE)
4	4 <input checked="" type="checkbox"/> ROTARY (AIR)
5	5 <input type="checkbox"/> AIR PERCUSSION
6	6 <input type="checkbox"/> BORING
7	7 <input type="checkbox"/> DIAMOND
8	8 <input type="checkbox"/> JETTING
9	9 <input type="checkbox"/> DRIVING

CONTRACTOR	
NAME OF WELL CONTRACTOR	LICENCE NUMBER
Shirley Hall Ltd.	2558
ADDRESS	
McDonalds Corners	
NAME OF DRILLER OR BORER	LICENCE NUMBER
Shirley Hall	2558
SIGNATURE OF CONTRACTOR	SUBMISSION DATE
Shirley Hall	DAY 29 MO 6 YR 83

OFFICE USE ONLY	
DATA SOURCE	CONTRACTOR
1	2558
DATE OF INSPECTION	INSPECTOR
REMARKS	



3507887

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COUNTY OR DISTRICT <i>South</i>	TOWNSHIP, BOROUGH, CITY, TOWN, VILLAGE <i>South Sherbrooke</i>	CON. BLOCK TRACT SURVEY, ETC. <i>7</i>	LOT <i>13</i>
83 <i>Maberly</i>			DATE COMPLETED DAY <i>14</i> MO <i>5</i> YR <i>87</i>
ELEVATION 11 12 13 14 15 16 17 18 19 20 21 22 23 24 25 26 27 28 29 30 31 32 33 34 35 36 37 38 39 40 41 42 43 44 45 46 47 48 49 50 51 52 53 54 55 56 57 58 59 60 61 62 63 64 65 66 67 68 69 70 71 72 73 74 75 76 77 78 79 80 81 82 83 84 85 86 87 88 89 90 91 92 93 94 95 96 97 98 99 100			

LOG OF OVERBURDEN AND BEDROCK MATERIALS (SEE INSTRUCTIONS)

[illegible]

31

32

1	2	10	14	15	21
41		WATER RECORD			
WATER FOUND AT - FEET		KIND OF WATER			
10-13		1 <input checked="" type="checkbox"/> FRESH	3 <input type="checkbox"/> SULPHUR	14	
		2 <input type="checkbox"/> SALTY	4 <input type="checkbox"/> MINERALS		
			6 <input type="checkbox"/> GAS		
15-18		1 <input type="checkbox"/> FRESH	3 <input type="checkbox"/> SULPHUR	19	
		2 <input type="checkbox"/> SALTY	4 <input type="checkbox"/> MINERALS		
			6 <input type="checkbox"/> GAS		
20-23		1 <input type="checkbox"/> FRESH	3 <input type="checkbox"/> SULPHUR	24	
		2 <input type="checkbox"/> SALTY	4 <input type="checkbox"/> MINERALS		
			6 <input type="checkbox"/> GAS		
25-28		1 <input type="checkbox"/> FRESH	3 <input type="checkbox"/> SULPHUR	29	
		2 <input type="checkbox"/> SALTY	4 <input type="checkbox"/> MINERALS		
			6 <input type="checkbox"/> GAS		
30-33		1 <input type="checkbox"/> FRESH	3 <input type="checkbox"/> SULPHUR	34	
		2 <input type="checkbox"/> SALTY	4 <input type="checkbox"/> MINERALS		
			6 <input type="checkbox"/> GAS		

51 CASING & OPEN HOLE RECORD				
INSIDE DIAM INCHES	MATERIAL	WALL THICKNESS INCHES	DEPTH - FEET	
			FROM	TO
6 ¹⁰⁻¹¹	1 <input checked="" type="checkbox"/> STEEL 2 <input checked="" type="checkbox"/> GALVANIZED 3 <input type="checkbox"/> CONCRETE 4 <input type="checkbox"/> OPEN HOLE 5 <input type="checkbox"/> PLASTIC	12 188	0'	22 ¹³⁻¹⁴
17-18	1 <input type="checkbox"/> STEEL 2 <input type="checkbox"/> GALVANIZED 3 <input type="checkbox"/> CONCRETE 4 <input type="checkbox"/> OPEN HOLE 5 <input type="checkbox"/> PLASTIC	19		20-21
24-25	1 <input type="checkbox"/> STEEL 2 <input type="checkbox"/> GALVANIZED 3 <input type="checkbox"/> CONCRETE 4 <input type="checkbox"/> OPEN HOLE 5 <input type="checkbox"/> PLASTIC	26		27-30

SCREEN	SIZE(S) OF OPENING (SLOT NO.)	31-33	DIAMETER	34-38	LENGTH	39-40
	INCHES			FEET		
	MATERIAL AND TYPE		DEPTH TO TOP OF SCREEN		41-44	50
					FEET	

61 PLUGGING & SEALING RECORD			
DEPTH SET AT - FEET		MATERIAL AND TYPE	(CEMENT GROUT LEAD PACKER, ETC.)
FROM	TO		
10-13	14-17		
18-21	22-25		
26-29	30-33	60	

71	PUMPING TEST METHOD		10	PUMPING RATE		11-14	DURATION OF PUMPING	
	1 <input checked="" type="checkbox"/> PUMP 2 <input checked="" type="checkbox"/> SAILER			7		15-18 GPM		30 17-18 MINS
	STATIC LEVEL	WATER LEVEL END OF PUMPING	23	WATER LEVELS DURING		1 <input type="checkbox"/> PUMPING 2 <input checked="" type="checkbox"/> RECOVERY		
	15 15-21 FEET	23-24 FEET	15 MINUTES 15-20 FEET	30 MINUTES 20-31 FEET	45 MINUTES 15 15-24 FEET	60 MINUTES 15 15-27 FEET		
	IF FLOWING GIVE RATE		30-31 GPM	PUMP INTAKE SET AT		WATER AT END OF TEST		42
RECOMMENDED PUMP TYPE			RECOMMENDED PUMP SETTING		43-45	RECOMMENDED PUMPING RATE		
<input type="checkbox"/> SHALLOW <input checked="" type="checkbox"/> DEEP			70		7		46-49 GPM	
50-53								

54	FINAL STATUS OF WELL		1 <input checked="" type="checkbox"/> WATER SUPPLY	3 <input type="checkbox"/> ABANDONED INSUFFICIENT SUPPLY
	2 <input type="checkbox"/> OBSERVATION WELL	4 <input type="checkbox"/> RECHARGE WELL	5 <input type="checkbox"/> ABANDONED POOR QUALITY	7 <input type="checkbox"/> UNFINISHED
55-56	WATER USE		1 <input checked="" type="checkbox"/> DOMESTIC	5 <input type="checkbox"/> COMMERCIAL
	3 <input type="checkbox"/> STOCK	3 <input type="checkbox"/> IRRIGATION	4 <input type="checkbox"/> INDUSTRIAL	6 <input type="checkbox"/> MUNICIPAL
57	METHOD OF CONSTRUCTION		1 <input checked="" type="checkbox"/> CABLE TOOL	6 <input type="checkbox"/> BORING
	2 <input type="checkbox"/> ROTARY (CONVENTIONAL)	3 <input type="checkbox"/> ROTARY (REVERSE)	4 <input type="checkbox"/> ROTARY (AIR)	7 <input type="checkbox"/> DIAMOND
	5 <input type="checkbox"/> AIR PERCUSSION		8 <input type="checkbox"/> JETTING	9 <input type="checkbox"/> DRIVING
			<input type="checkbox"/> DIGGING	<input type="checkbox"/> OTHER


LOCATION OF WELL

IN DIAGRAM BELOW SHOW DISTANCES OF WELL FROM ROAD AND LOT LINE INDICATE NORTH BY ARROW

10851

DRILLERS REMARKS

CONTRACTOR	NAME OF WELL CONTRACTOR		WELL CONTRACTOR'S LICENCE NUMBER	
	Guy Hall Ltd		2558	
	ADDRESS			
	McDonalds Corners Ont.			
	NAME OF WELL TECHNICIAN		WELL TECHNICIAN'S LICENCE NUMBER	
	Brant Echlin		10273	
	SIGNATURE OF TECHNICIAN/CONTRACTOR		SUBMISSION DATE	
	Guy Hall		DAY 10 MO 6 YR 87	

OFFICE USE ONLY	DATA SOURCE	58 CONTRACTOR	59-62	DATE RECEIVED	63-68	80
				JUN 18 1987		
	DATE OF INSPECTION		INSPECTOR			
REMARKS						
<div style="text-align: right;">  </div>						

Print only in spaces provided.
Mark correct box with a checkmark, where applicable.

11

3513257

Municipality **35014** Con. **CON** **06**

County or District <i>Lamp</i>	Township/Borough/City/Town/Village <i>South Sherbrooke</i>	Con block tract survey, etc. 6	Lot 13
Address <i>596 North Russell Rd., Russell</i>		Date completed 13 day 3 month year 01	
U M	Northing	RC Elevation <i>849</i>	Bath code ii iii iv
21			

LOG OF OVERBURDEN AND BEDROCK MATERIALS (see instructions)					
General colour	Most common material	Other materials	General description	Depth - feet	
				From	To
red	sand			0'	1 1/2'
black	granite			1 1/2'	23'
grey/orange/green	granite			23'	130'
black/grey	granite			130'	156'
grey/orange	granite			156'	190'
black	granite			190'	200'

[illegible]

41 WATER RECORD			
Water found at - feet	Kind of water		
70 ¹⁰⁻¹³	1 <input checked="" type="checkbox"/> Fresh 2 <input type="checkbox"/> Salty	3 <input type="checkbox"/> Sulphur 4 <input type="checkbox"/> Minerals 6 <input type="checkbox"/> Gas	14
15-18	1 <input type="checkbox"/> Fresh 2 <input type="checkbox"/> Salty	3 <input type="checkbox"/> Sulphur 4 <input type="checkbox"/> Minerals 6 <input type="checkbox"/> Gas	19
20-23	1 <input type="checkbox"/> Fresh 2 <input type="checkbox"/> Salty	3 <input type="checkbox"/> Sulphur 4 <input type="checkbox"/> Minerals 6 <input type="checkbox"/> Gas	24
25-28	1 <input type="checkbox"/> Fresh 2 <input type="checkbox"/> Salty	3 <input type="checkbox"/> Sulphur 4 <input type="checkbox"/> Minerals 6 <input type="checkbox"/> Gas	29
30-33	1 <input type="checkbox"/> Fresh 2 <input type="checkbox"/> Salty	3 <input type="checkbox"/> Sulphur 4 <input type="checkbox"/> Minerals 6 <input type="checkbox"/> Gas	34

51 CASING & OPEN HOLE RECORD				
Inside diam inches	Material	Wall thickness inches	Depth - feet	
			From	To
6 ¹⁰⁻¹³	1 <input checked="" type="checkbox"/> Steel 2 <input type="checkbox"/> Galvanized 3 <input type="checkbox"/> Concrete 4 <input type="checkbox"/> Open hole 5 <input type="checkbox"/> Plastic	188	0'	22 ¹³⁻¹⁶
17-18	1 <input type="checkbox"/> Steel 2 <input type="checkbox"/> Galvanized 3 <input type="checkbox"/> Concrete 4 <input type="checkbox"/> Open hole 5 <input type="checkbox"/> Plastic			20-23
24-25	1 <input type="checkbox"/> Steel 2 <input type="checkbox"/> Galvanized 3 <input type="checkbox"/> Concrete 4 <input type="checkbox"/> Open hole 5 <input type="checkbox"/> Plastic			27-30

SCREEN	61 PLUGGING & SEALING RECORD		
	Sizes of opening (Slot No.)		Diameter
	Inches		feet
	Material and type		Depth at top of screen
			feet

61 PLUGGING & SEALING RECORD			
Depth set at - feet		Material and type (Cement grout, bentonite, etc.)	
From	To		
0 ¹⁰⁻¹³	22 ¹⁴⁻¹⁷	Cement	
18-21	22-25		
26-29	30-33		

PUMPING TEST	71 Pumping test method ¹⁰ <input checked="" type="checkbox"/> Pump <input type="checkbox"/> Bailer		Pumping rate ⁴ ¹¹⁻¹⁴ ¹⁵⁻¹⁶ ¹⁷⁻¹⁸ 47 GPM		Duration of pumping 1 ¹⁵⁻¹⁶ ¹⁷⁻¹⁸ Hours	
	Static level	Water level end of pumping	25 Water levels during 1 <input checked="" type="checkbox"/> Pumping 2 <input type="checkbox"/> Recovery			
	31' ¹⁶⁻²¹ feet	22-24 feet	15 minutes ²⁵⁻²⁹ 135' feet	30 minutes ³⁰⁻³¹ 70' feet	45 minutes ³²⁻³⁴ 42' feet	60 minutes ³⁵⁻³⁷ 31' feet
	If flowing give rate ³⁸⁻⁴¹ GPM	Pump intake set at 200 feet		Water at end of test ⁴² <input type="checkbox"/> Clear <input checked="" type="checkbox"/> Cloudy		
	Recommended pump type <input type="checkbox"/> Shallow <input checked="" type="checkbox"/> Deep	Recommended pump setting ⁴³⁻⁴⁵ 175' feet		Recommended pump rate ⁴⁶⁻⁴⁸ 4 GPM		

FINAL STATUS OF WELL			54
1 <input checked="" type="checkbox"/> Water supply	5 <input type="checkbox"/> Abandoned, insufficient supply	9 <input type="checkbox"/> Unfinished	
2 <input type="checkbox"/> Observation well	6 <input type="checkbox"/> Abandoned, poor quality	10 <input type="checkbox"/> Replacement well	
3 <input type="checkbox"/> Test hole	7 <input type="checkbox"/> Abandoned (Other)		
4 <input type="checkbox"/> Recharge well	8 <input type="checkbox"/> Dewatering		

WATER USE			55-56
1 <input checked="" type="checkbox"/> Domestic	5 <input type="checkbox"/> Commercial	9 <input type="checkbox"/> Not use	
2 <input type="checkbox"/> Stock	6 <input type="checkbox"/> Municipal	10 <input type="checkbox"/> Other	
3 <input type="checkbox"/> Irrigation	7 <input type="checkbox"/> Public supply		
4 <input type="checkbox"/> Industrial	8 <input type="checkbox"/> Cooling & air conditioning		

METHOD OF CONSTRUCTION			57
1 <input type="checkbox"/> Cable tool	5 <input type="checkbox"/> Air percussion	9 <input type="checkbox"/> Driving	
2 <input type="checkbox"/> Rotary (conventional)	6 <input type="checkbox"/> Boring	10 <input type="checkbox"/> Logging	
3 <input type="checkbox"/> Rotary (reverse)	7 <input type="checkbox"/> Diamond	11 <input type="checkbox"/> Other	
4 <input checked="" type="checkbox"/> Rotary (air)	8 <input type="checkbox"/> Jetting		

LOCATION OF WELL

In diagram below show distances of well from road and lot line.
Indicate north by arrow.

225789

Name of Well Contractor <i>Yuef Hael Ltd</i>	Well Contractor's Licence No. <i>2558</i>
Address <i>RR1 McDonalds Corners Ont K0G1M0</i>	
Name of Well Technician <i>Mark Hael</i>	Well Technician's Licence No. <i>T2228</i>
Signature of Technician/Contractor <i>Mark Hael</i>	Submission date <i>15</i> day <i>3</i> mo <i>01</i> yr

MINISTRY USE ONLY	Data source	58	Contractor	59-62	Date received	63-66	67
			2558		APR 11 2001		
	Date of inspection		Inspector				
	Remarks						
	CSS.ES1						



Ministry
of the
Environment

The Ontario Water Resources Act
WATER WELL RECORD

Print only in spaces provided.
Mark correct box with a checkmark, where applicable.

11

3513786

Municipality Con. 35014 CON 12

County or District Lanark	Township/Borough/City/Town/Village South Sherbrooke	Con block tract survey, etc. 12	Lot 4
Owner's surname [REDACTED]	First Name [REDACTED]	Address Box 1456, Kemptonville Ont.	
Date completed 17 7 02		day month year	
Zone 21	Easting 10	Northing 17	RC K06159

LOG OF OVERBURDEN AND BEDROCK MATERIALS (see instructions)

General colour	Most common material	Other materials	General description	Depth - feet	
				From	To
	sand/stones			0'	6'
	black/grey granite			6'	35'
	black/red granite			35'	38'
	black/grey granite			38'	75'
	black/red granite			75'	114'
	black/pink/grey granite			114'	220'

31	32
----	----

WATER RECORD	
Water found at - feet	Kind of water
92' 10-13	1 <input checked="" type="checkbox"/> Fresh 4 <input type="checkbox"/> Sulphur 14
235' 16-18	2 <input type="checkbox"/> Salty 6 <input type="checkbox"/> Minerals 19
	3 <input type="checkbox"/> Gas 6 <input type="checkbox"/> Gas 19
20-23	1 <input type="checkbox"/> Fresh 3 <input type="checkbox"/> Sulphur 24
	2 <input type="checkbox"/> Salty 4 <input type="checkbox"/> Minerals 24
25-28	1 <input type="checkbox"/> Fresh 3 <input type="checkbox"/> Sulphur 29
	2 <input type="checkbox"/> Salty 4 <input type="checkbox"/> Minerals 29
30-33	1 <input type="checkbox"/> Fresh 3 <input type="checkbox"/> Sulphur 34
	2 <input type="checkbox"/> Salty 4 <input type="checkbox"/> Minerals 34

CASING & OPEN HOLE RECORD	
Inside diam inches	Material
6" 11	1 <input checked="" type="checkbox"/> Steel 12
	2 <input type="checkbox"/> Galvanized 12
	3 <input type="checkbox"/> Concrete 12
	4 <input type="checkbox"/> Open hole 12
	5 <input type="checkbox"/> Plastic 12
17-18	1 <input type="checkbox"/> Steel 18
	2 <input type="checkbox"/> Galvanized 18
	3 <input type="checkbox"/> Concrete 18
	4 <input type="checkbox"/> Open hole 18
	5 <input type="checkbox"/> Plastic 18
24-25	1 <input type="checkbox"/> Steel 26
	2 <input type="checkbox"/> Galvanized 26
	3 <input type="checkbox"/> Concrete 26
	4 <input type="checkbox"/> Open hole 26
	5 <input type="checkbox"/> Plastic 26

Sizes of opening (Slot No.)	Diameter	Length
	inches	feet
Material and type	Depth at top of screen	
	feet	

PLUGGING & SEALING RECORD	
<input type="checkbox"/> Annular space	<input type="checkbox"/> Abandonment
Depth set at - feet	Material and type (Cement grout, bentonite, etc.)
From To	
0' 23' 47	Cement
18-21	22-25
26-29	30-33

Pumping test method	Pumping rate	Duration of pumping
1 <input checked="" type="checkbox"/> Pump 2 <input type="checkbox"/> Bailer	3 GPM	17-18 Hours Mins
Static level	Water level end of pumping	Water levels during
16 feet	22-24 feet	15 minutes 175' 30 minutes 130' 45 minutes 85' 60 minutes 45'
If flowing give rate	Pump intake set at	Water at end of test
GPM	220 feet	<input type="checkbox"/> Clear <input checked="" type="checkbox"/> Cloudy
Recommended pump type	Recommended pump setting	Recommended pump rate
<input type="checkbox"/> Shallow <input checked="" type="checkbox"/> Deep	190 feet	3 GPM

FINAL STATUS OF WELL	
1 <input checked="" type="checkbox"/> Water supply	5 <input type="checkbox"/> Abandoned, insufficient supply
2 <input type="checkbox"/> Observation well	6 <input type="checkbox"/> Abandoned, poor quality
3 <input type="checkbox"/> Test hole	7 <input type="checkbox"/> Abandoned (Other)
4 <input type="checkbox"/> Recharge well	8 <input type="checkbox"/> Dewatering
9 <input type="checkbox"/> Unfinished	10 <input type="checkbox"/> Replacement well
WATER USE	
1 <input checked="" type="checkbox"/> Domestic	5 <input type="checkbox"/> Commercial
2 <input type="checkbox"/> Stock	6 <input type="checkbox"/> Municipal
3 <input type="checkbox"/> Irrigation	7 <input type="checkbox"/> Public supply
4 <input type="checkbox"/> Industrial	8 <input type="checkbox"/> Cooling & air conditioning
9 <input type="checkbox"/> Not use	10 <input type="checkbox"/> Other
METHOD OF CONSTRUCTION	
1 <input type="checkbox"/> Cable tool	5 <input type="checkbox"/> Air percussion
2 <input type="checkbox"/> Rotary (conventional)	6 <input type="checkbox"/> Boring
3 <input type="checkbox"/> Rotary (reverse)	7 <input type="checkbox"/> Diamond
4 <input checked="" type="checkbox"/> Rotary (air)	8 <input type="checkbox"/> Jetting
9 <input type="checkbox"/> Driving	10 <input type="checkbox"/> Digging
11 <input type="checkbox"/> Other	

LOCATION OF WELL

In diagram below show distances of well from road and lot line. Indicate north by arrow.

Silvery lane

10' from garage

Little Silver Lake

246248

Name of Well Contractor Huf Hall Ltd	Well Contractor's Licence No. 2558
Address RR1 Mc Donalds Corners Ont K0G1M0	
Name of Well Technician Mark Hall	Well Technician's Licence No. T2228
Signature of Technician/Contractor Huf Hall	Submission date 18 7 02
	day mo yr

MINISTRY USE ONLY	Data source 2558	Contractor 2558	Date received AUG 13 2002
	Date of inspection	Inspector	
	Remarks		



Instructions for Completing Form

- For use in the Province of Ontario only. This document is a permanent legal document. Please retain for future reference.
- All Sections must be completed in full to avoid delays in processing. Further instructions and explanations are available on the back of this form.
- Questions regarding completing this application can be directed to the Water Well Management Coordinator at 416-235-6203.
- All metre measurements shall be reported to 1/10th of a metre.
- Please print clearly in blue or black ink only.

Ministry Use Only

Address of Well Location (County/District/Municipality) 133 Rainbow Lane, Lanark Cty, South Sherbrooke Twp
RR#/Street Number/Name South Sherbrooke Twp
GPS Reading NAD Zone Easting Northing Unit Make/Model Mode of Operation: Undifferentiated Averaged
8 3 18 381292 4962789 Magellan WASS

Log of Overburden and Bedrock Materials (see instructions)

General Colour	Most common material	Other Materials	General Description	Depth From	Metres To
	sand/gravel/stones			0.	2.13
	black/pink granite			2.13	14.33
	black/grey/green (mica) granite			14.33	15.85
	black granite			15.85	21.95
	white/grey granite			21.95	27.43
	black/pink granite			27.43	41.15
	black granite			41.15	42.67

4 gpm @ 140 feet

Hole Diameter	Construction Record	Test of Well Yield
Depth From Metres To Centimetres 0. 6.70 25.4	Inside diam centimetres Material Wall thickness centimetres Depth From Metres To 15.24 Steel Fibreglass .48 0.61 6.70 Plastic Concrete Galvanized Steel Fibreglass Plastic Concrete Galvanized Steel Fibreglass Plastic Concrete Galvanized Screen Outside diam Steel Fibreglass Slot No. Plastic Concrete Galvanized No Casing or Screen Open hole	Pumping test method Draw Down Recovery Pump. Time min Water Level Metres Time min Water Level Metres Pump intake set at - (metres) 24.38 Static Level Pumping rate - (litres/min) 13.18 1 7.92 1 10.06 Duration of pumping 1 hrs + min 2 8.19 2 9.75 Final water level end of pumping 6.90 metres 3 8.32 3 9.53 Recommended pump type 4 8.43 4 9.37 Recommended pump depth 35 metres 5 8.52 5 9.25 Recommended pump rate 18 (litres/min) 10 8.90 10 8.88 If flowing give rate - 8 (litres/min) 15 9.08 15 8.63 20 9.56 20 8.50 25 9.85 25 8.37 If pumping discontinued, give reason. 30 10.15 30 8.35 N/A 40 10.43 40 8.08 50 10.71 50 7.90 60 10.95 60 7.74

Plugging and Sealing Record	Annular space	Abandonment
Depth set at - Metres From To 6.70 0. Quick grout 0.22		

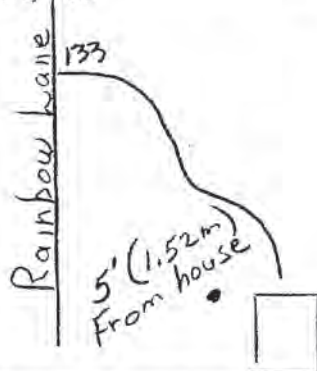
Method of Construction
<input type="checkbox"/> Cable Tool <input checked="" type="checkbox"/> Rotary (air) <input type="checkbox"/> Diamond <input type="checkbox"/> Digging <input type="checkbox"/> Rotary (conventional) <input type="checkbox"/> Air percussion <input type="checkbox"/> Jetting <input type="checkbox"/> Other <input type="checkbox"/> Rotary (reverse) <input type="checkbox"/> Boring <input type="checkbox"/> Driving

Water Use
<input checked="" type="checkbox"/> Domestic <input type="checkbox"/> Industrial <input type="checkbox"/> Public Supply <input type="checkbox"/> Other <input type="checkbox"/> Stock <input type="checkbox"/> Commercial <input type="checkbox"/> Not used <input type="checkbox"/> Irrigation <input type="checkbox"/> Municipal <input type="checkbox"/> Cooling & air conditioning

Final Status of Well
<input checked="" type="checkbox"/> Water Supply <input type="checkbox"/> Recharge well <input type="checkbox"/> Unfinished <input type="checkbox"/> Abandoned, (Other) <input type="checkbox"/> Observation well <input type="checkbox"/> Abandoned, insufficient supply <input type="checkbox"/> Dewatering <input type="checkbox"/> Test Hole <input type="checkbox"/> Abandoned, poor quality <input type="checkbox"/> Replacement well

Well Contractor/Technician Information
Name of Well Contractor WINE HALL Ltd Well Contractor's Licence No. 2558 Business Address (street name, number, city etc.) RR1, 260 Hall Shore Rd McDonald's Corners Ont K0G1M0 Name of Well Technician (last name, first name) Mark Hall Well Technician's Licence No. 2558 Signature of Technician/Contractor x Mark Hall Date Submitted 2004 03 23

Location of Well
In diagram below show distances of well from road, lot line, and building. Indicate north by arrow.



Audit No. Z 06648 Date Well Completed 2004 03 23	Was the well owner's information package delivered? Yes No	Date Delivered 2004 03
--	--	------------------------

Ministry Use Only
Data Source Contractor 2558 Date Received APR 16 2004 Date of Inspection Remarks Well Record Number CSS.E55 3514498

Instructions for Completing Form

- For use in the **Province of Ontario** only. This document is a permanent **legal** document. Please retain for future reference.
- All Sections **must** be completed in full to avoid delays in processing. Further instructions and explanations are available on the back of this form.
- Questions regarding completing this application can be directed to the Water Well Help Desk (Toll Free) at 1-888-396-9355.
- All metre measurements shall be reported to 1/10th of a metre.**
- Please print clearly in blue or black ink only.

Ministry Use Only

Address of Well Location (County/District/Municipality) # 4193 Hwy #36 Township South Sherbrooke Lot 13 Concession 6
RR#/Street Number/Name Con 6 Lot 13 City/Town/Village _____ Site/Compartment/Block/Tract etc. _____
GPS Reading NAD 83 Zone 18 Easting 3810249 Northing 491627316 Unit Make/Model Magellan Mode of Operation: ☐ Undifferentiated ☒ Averaged
☐ Differentiated, specify _____

Log of Overburden and Bedrock Materials (see instructions)

General Colour	Most common material	Other Materials	General Description	Depth From	Metres To
red	Sand			0.	0.91
black/red	granite			0.91	32.92
black/brown	granite			32.92	33.53
black	granite			33.53	35.05
10 gpm (45.46 litres) 115' Deep (35.05m)					

Hole Diameter

Depth From	Metres To	Diameter Centimetres
0.	6.70	25.4

Water Record

Water found at 32.92m Kind of Water ☒ Fresh ☐ Sulphur ☐ Gas ☐ Salty ☐ Minerals ☒ Other: not tested

☐ m ☐ Fresh ☐ Sulphur ☐ Gas ☐ Salty ☐ Minerals ☐ Other: _____

After test of well yield, water was ☐ Clear and sediment free ☒ Other, specify cloudy

Chlorinated ☒ Yes ☐ No

Construction Record

Inside diam centimetres	Material	Wall thickness centimetres	Depth From	Metres To
1524	<input checked="" type="checkbox"/> Steel <input type="checkbox"/> Fibreglass <input type="checkbox"/> Plastic <input type="checkbox"/> Concrete <input type="checkbox"/> Galvanized	.48	0.61	35.05

Casing

☐ Steel ☐ Fibreglass ☐ Plastic ☐ Concrete ☐ Galvanized

Screen

Outside diam ☐ Steel ☐ Fibreglass ☐ Plastic ☐ Concrete ☐ Galvanized Slot No. _____

No Casing or Screen

☒ Open hole 6.70 35.05

Test of Well Yield

Pumping test method	Draw Down Time min	Water Level Metres	Recovery Time min	Water Level Metres
<u>Pump</u>				
Pump intake set at - (metres) <u>30</u>				
Pumping rate - (litres/min) <u>45</u>	1	10.60	1	21.25
Duration of pumping <u>1</u> hrs + <u>0</u> min	2	11.75	2	19.09
Final water level end of pumping <u>11.43</u> metres	3	12.70	3	17.80
Recommended pump type <input type="checkbox"/> Shallow <input checked="" type="checkbox"/> Deep	4	13.28	4	16.78
Recommended pump depth <u>27</u> metres	5	13.90	5	15.85
Recommended pump rate <u>35</u> (litres/min)	10	16.37	10	13.33
If flowing give rate - (litres/min) <u>28</u>	15	17.94	15	11.92
	20	19.30	20	11.10
	25	20.09	25	10.48
If pumping discontinued, give reason.	30	21.90	30	10.04
	40	23.91	40	9.28
	50	25.08	50	8.74
	60	25.75	60	8.33

Plugging and Sealing Record ☒ Annular space ☐ Abandonment

Depth set at - Metres From	To	Material and type (bentonite slurry, neat cement slurry) etc.	Volume Placed (cubic metres)
6.70	0.	2 Bags cement	0.044
		2 Bags quick grout	0.044

Method of Construction

☐ Cable Tool ☒ Rotary (air) ☐ Diamond ☐ Digging ☐ Rotary (conventional) ☐ Air percussion ☐ Jetting ☐ Other ☐ Rotary (reverse) ☐ Boring ☐ Driving

Water Use

☒ Domestic ☐ Industrial ☐ Public Supply ☐ Other ☐ Stock ☐ Commercial ☐ Not used ☐ Irrigation ☐ Municipal ☐ Cooling & air conditioning

Final Status of Well

☒ Water Supply ☐ Recharge well ☐ Unfinished ☐ Abandoned, (Other) ☐ Observation well ☐ Abandoned, insufficient supply ☐ Dewatering ☐ Test Hole ☐ Abandoned, poor quality ☐ Replacement well

Well Contractor/Technician Information

Name of Well Contractor WILF Hall & Sons Well Drilling RR1 Well Contractor's Licence No. 2558
Business Address (street name, number, city etc.) 260 Hall Shore Rd, McDonald's Corners Ont K0G1M0
Name of Well Technician (last name, first name) Mark Hall Well Technician's Licence No. 73228
Signature of Technician/Contractor x Mark Hall Date Submitted 2007 17 14

Location of Well

In diagram below show distances of well from road, lot line, and building. Indicate north by arrow.

Hwy #36
#4193
Well
Pond

Audit No. 2 58533 Date Well Completed 2007 17 14
Was the well owner's information package delivered? ☒ Yes ☐ No Date Delivered 2007 17 14

Ministry Use Only

Data Source _____ Contractor 2558
Date Received Aug 17 2007 Date of Inspection Aug 17 2007
Remarks _____ Well Record Number _____

Well Location

Address of Well Location (Street Number/Name) 4417 County Rd 36		Township South Sherbrooke	Lot 13	Concession 7
County/District/Municipality Lanark County		City/Town/Village Maberly	Province Ontario	Postal Code
UTM Coordinates NAD 83	Zone 18	Easting 379404	Northings 4963543	Municipal Plan and Sublot Number Other

Overburden and Bedrock Materials/Abandonment Sealing Record (see instructions on the back of this form)

General Colour	Most Common Material	Other Materials	General Description	Depth (m/ft) From	To
black	earth			0	.6
red & black	granite			.6	19.8
green	granite			19.8	42.7

Annular Space			
Depth Set at (m/ft) From	To	Type of Sealant Used (Material and Type)	Volume Placed (m³/ft³)
0	6.1	Cement	120kg

Method of Construction	Well Use
<input type="checkbox"/> Cable Tool <input checked="" type="checkbox"/> Rotary (Conventional) <input type="checkbox"/> Rotary (Reverse) <input type="checkbox"/> Boring <input type="checkbox"/> Air percussion <input type="checkbox"/> Other, specify	<input type="checkbox"/> Public <input type="checkbox"/> Domestic <input type="checkbox"/> Livestock <input type="checkbox"/> Irrigation <input type="checkbox"/> Industrial <input type="checkbox"/> Other, specify
<input type="checkbox"/> Diamond <input type="checkbox"/> Jetting <input type="checkbox"/> Driving <input type="checkbox"/> Digging	<input type="checkbox"/> Commercial <input type="checkbox"/> Municipal <input type="checkbox"/> Test Hole <input type="checkbox"/> Cooling & Air Conditioning <input type="checkbox"/> Not used <input type="checkbox"/> Dewatering <input type="checkbox"/> Monitoring

Construction Record - Casing				Status of Well	
Inside Diameter (cm/in)	Open Hole OR Material (Galvanized, Fibreglass, Concrete, Plastic, Steel)	Wall Thickness (cm/in)	Depth (m/ft) From	To	
15.85	steel	.88	0	6.1	<input checked="" type="checkbox"/> Water Supply <input type="checkbox"/> Replacement Well <input type="checkbox"/> Test Hole <input type="checkbox"/> Recharge Well <input type="checkbox"/> Dewatering Well <input type="checkbox"/> Observation and/or Monitoring Hole <input type="checkbox"/> Alteration (Construction) <input type="checkbox"/> Abandoned, Insufficient Supply <input type="checkbox"/> Abandoned, Poor Water Quality <input type="checkbox"/> Abandoned, other, specify <input type="checkbox"/> Other, specify

Construction Record - Screen				Status of Well	
Outside Diameter (cm/in)	Material (Plastic, Galvanized, Steel)	Slot No.	Depth (m/ft) From	To	

Water Details		Hole Diameter	
Water found at Depth 12 (m/ft)	Kind of Water: <input type="checkbox"/> Fresh <input type="checkbox"/> Untested <input type="checkbox"/> Gas <input type="checkbox"/> Other, specify	Depth (m/ft) From	To
Water found at Depth 36 (m/ft)	Kind of Water: <input type="checkbox"/> Fresh <input type="checkbox"/> Untested <input type="checkbox"/> Gas <input type="checkbox"/> Other, specify	6.1	42.7
Water found at Depth (m/ft)	Kind of Water: <input type="checkbox"/> Fresh <input type="checkbox"/> Untested <input type="checkbox"/> Gas <input type="checkbox"/> Other, specify		

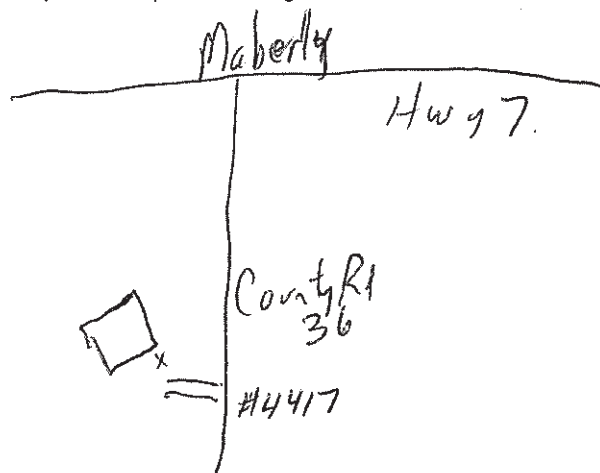
Business Name of Well Contractor J R Thompson		Well Contractor's Licence No. 49015	
Business Address (Street Number/Name) 2076 Old Brook Rd		Municipality Maberly	
Province ON	Postal Code K0H 1B0	Business E-mail Address	
Bus. Telephone No. (inc. area code) 613 267 4890		Name of Well Technician (Last Name, First Name) Darrell Stevenson	
Well Technician's Licence No. 2919		Signature of Technician and/or Contractor Briann Brady	
Date Submitted 2010/1/25			

Results of Well Yield Testing

After test of well yield, water was:		Draw Down		Recovery	
<input checked="" type="checkbox"/> Clear and sand free	<input type="checkbox"/> Other, specify	Time (min)	Water Level (m/ft)	Time (min)	Water Level (m/ft)
If pumping discontinued, give reason:		Static Level	7.3		
Pump intake set at (m/ft) 40		1	8.5	1	10.2
Pumping rate (l/min / GPM) 27		2	8.7	2	9.4
Duration of pumping 1 hrs + min		3	9.0	3	9.2
Final water level end of pumping (m/ft) 11.4		4	9.2	4	9.0
If flowing give rate (l/min / GPM)		5	9.4	5	8.8
Recommended pump depth (m/ft) 39		10	9.8	10	8.3
Recommended pump rate (l/min / GPM) 25		15	10.2	15	7.9
Well production (l/min / GPM) 26		20	10.6	20	7.6
Disinfected? <input checked="" type="checkbox"/> Yes <input type="checkbox"/> No		25	10.8	25	7.3
		30	11.0	30	7.3
		40	11.2	40	7.3
		50	11.3	50	7.3
		60	11.4	60	7.3

Map of Well Location

Please provide a map below following instructions on the back.



Comments:

Well owner's information package delivered <input checked="" type="checkbox"/> Yes <input type="checkbox"/> No	Date Package Delivered 2010/1/25 Date Work Completed 2010/1/25	Ministry Use Only Audit No. 2124218 Received JAN 24 2011
--	---	---

Address of Well Location (Street Number/Name) 4452 Bolingbrooke Road		Township South Sherbrooke	Lot 13	Concession 7
County/District/Municipality Lanark		City/Town/Village Maberly	Province Ontario	Postal Code K0H2B0
UTM Coordinates NAD 83	Zone 18	Easting 3794284	Northings 963656	Municipal Plan and Sublot Number /

Overburden and Bedrock Materials/Abandonment Sealing Record (see instructions on the back of this form)					
General Colour	Most Common Material	Other Materials	General Description	Depth (m/ft)	
Red	Sand/stones			0'	4'
black/grey/red	granite			4'	32'
black/grey	granite			32'	198'
grey	granite			198'	205'
black	granite			205'	280'
Red	granite			280'	284'
Black/grey	granite			284'	310'
Black/grey/pink	granite			310'	400'

Annular Space			
Depth Set at (m/ft)	Type of Sealant Used (Material and Type)	Volume Placed (m³/ft³)	
0' 22'	2 Bags cement	0.044	
	2 Bags quick grout	0.044	

Method of Construction		Well Use		
<input type="checkbox"/> Cable Tool	<input type="checkbox"/> Diamond	<input type="checkbox"/> Public	<input type="checkbox"/> Commercial	<input type="checkbox"/> Not used
<input checked="" type="checkbox"/> Rotary (Conventional)	<input type="checkbox"/> Jetting	<input checked="" type="checkbox"/> Domestic	<input type="checkbox"/> Municipal	<input type="checkbox"/> Dewatering
<input type="checkbox"/> Rotary (Reverse)	<input type="checkbox"/> Driving	<input type="checkbox"/> Livestock	<input checked="" type="checkbox"/> Test Hole	<input type="checkbox"/> Monitoring
<input type="checkbox"/> Boring	<input type="checkbox"/> Digging	<input type="checkbox"/> Irrigation	<input type="checkbox"/> Cooling & Air Conditioning	
<input type="checkbox"/> Air percussion		<input type="checkbox"/> Industrial		
<input type="checkbox"/> Other, specify		<input type="checkbox"/> Other, specify		

Construction Record - Casing				Status of Well	
Inside Diameter (cm/in)	Open Hole OR Material (Galvanized, Fibreglass, Concrete, Plastic, Steel)	Wall Thickness (cm/in)	Depth (m/ft)		
6"	Steel	.48cm	0' 22'	<input checked="" type="checkbox"/> Water Supply	
				<input type="checkbox"/> Replacement Well	
				<input type="checkbox"/> Test Hole	
				<input type="checkbox"/> Recharge Well	
				<input type="checkbox"/> Dewatering Well	
				<input type="checkbox"/> Observation and/or Monitoring Hole	
				<input type="checkbox"/> Alteration (Construction)	
				<input type="checkbox"/> Abandoned, Insufficient Supply	
				<input type="checkbox"/> Abandoned, Poor Water Quality	
				<input type="checkbox"/> Abandoned, other, specify	
				<input type="checkbox"/> Other, specify	

Construction Record - Screen					
Outside Diameter (cm/in)	Material (Plastic, Galvanized, Steel)	Slot No.	Depth (m/ft)		
			From To		

Water Details		Hole Diameter	
Water found at Depth (m/ft) <input type="checkbox"/> Gas <input type="checkbox"/> Other, specify	Kind of Water: <input type="checkbox"/> Fresh <input checked="" type="checkbox"/> Untested	Depth (m/ft)	Diameter (cm/in)
Water found at Depth (m/ft) <input type="checkbox"/> Gas <input type="checkbox"/> Other, specify	Kind of Water: <input type="checkbox"/> Fresh <input type="checkbox"/> Untested	0' 22'	25.4cm
Water found at Depth (m/ft) <input type="checkbox"/> Gas <input type="checkbox"/> Other, specify	Kind of Water: <input type="checkbox"/> Fresh <input type="checkbox"/> Untested		

Well Contractor and Well Technician Information			
Business Name of Well Contractor WILF HALL & SONS WELL DRILLING		Well Contractor's Licence No. 2151518	
Business Address (Street Number/Name) 256 Hall Shore Rd BR1 McDonald's Corners		Municipality	
Province ON	Postal Code K0G1M0	Business E-mail Address wilfhall1td@bellnet.ca	
Bus. Telephone No. (inc. area code) 6132782933		Name of Well Technician (Last Name, First Name) Hall Mark	
Well Technician's Licence No. 121228		Signature of Technician and/or Contractor Mark Hall	
		Date Submitted 2012/09/24	

Results of Well Yield Testing			
After test of well yield, water was:		Draw Down	
<input type="checkbox"/> Clear and sand free		Time (min)	Water Level (m/ft)
<input checked="" type="checkbox"/> Other, specify Cloudy		Static Level	
If pumping discontinued, give reason:		1	21.0'
Pump intake set at (m/ft)		2	23.4'
Pumping rate (l/min / GPM)		3	26.0'
Duration of pumping		4	28.8'
1 hrs + min		5	31.3'
Final water level end of pumping (m/ft)		10	43.25'
If flowing give rate (l/min / GPM)		15	55.0'
Recommended pump depth (m/ft)		20	65.85'
350'		25	76.2'
Recommended pump rate (l/min / GPM)		30	86.2'
2 gpm		40	104.65'
Well production (l/min / GPM)		50	124.6'
1 1/2 gpm		60	142.5'
Disinfected?			
<input checked="" type="checkbox"/> Yes <input type="checkbox"/> No			

Map of Well Location										
Please provide a map below following instructions on the back.										
<table border="1"> <tr> <th>Well owner's information package delivered</th><th>Date Package Delivered</th><th>Ministry Use Only</th></tr> <tr> <td><input checked="" type="checkbox"/> Yes</td><td>2012/09/24</td><td>Audit No. Z153980</td></tr> <tr> <td><input type="checkbox"/> No</td><td>2012/09/24</td><td>Received OCT 05 2012</td></tr> </table>		Well owner's information package delivered	Date Package Delivered	Ministry Use Only	<input checked="" type="checkbox"/> Yes	2012/09/24	Audit No. Z153980	<input type="checkbox"/> No	2012/09/24	Received OCT 05 2012
Well owner's information package delivered	Date Package Delivered	Ministry Use Only								
<input checked="" type="checkbox"/> Yes	2012/09/24	Audit No. Z153980								
<input type="checkbox"/> No	2012/09/24	Received OCT 05 2012								

APPENDIX E

Well Owner Interview Forms

DRAFT





WELL INSPECTION SURVEY FORM

WELL ID (SAMPLE ID): 7046732 Sample Date/Time:
Blind Duplicate ID:

OWNER/ADDRESS OF WELL: Sample Location:
Sampler Initials:

Name: Karen Prytula Person Interviewed: Matt Debeer

Address: County Rd 36, Con 6, lot 13

Telephone (Home): (Business):

How Long as Owner: Heating: Oil Gas Electric Other (circle one)

Sampling Results Requested? Yes or No E-mail Address:

Field Readings: Turbidity Temp °C pH TDS EC

TENANT (if different from owner):

Name: Person Interviewed:

Telephone (Home): (Business):

How Long as Tenant:

PART I: PREVIOUS WELL ISSUES

Type of Water Quality Issue:

Type of Water Quantity Issue:

Outcome(s):

Available Documentation:

Available Sampling Results?:

PART II: WELL CONSTRUCTION DETAILS

No. of Wells/Type (dug/drilled): Drilled

Does well draw water from overburden or bedrock? (circle correct one)

Location of Well in Relation to Residence/Buildings: 1

Well 1 GPS coordinates: (N) 4963628 (E) 379621

Well 2 GPS coordinates: (N) (E)

Water Well Record Available?: Y (Y/N; attach copy) Construction Date: 2007/06/28

Well Type (dug, drilled): Drilled

Well Depth (m): 67.07 Diameter (cm): 25.4

Casing Length (m): 6.70 Diameter (cm): 15.24

Screen Installed? N (Y/N) Water Quality: "Best water in the world"

Test Pumping Rate: 13.64 L/min Test Max. Drawdown/Time:

Depth to Bedrock (m): 1.52 Bedrock Decript: Black/grey granite

Depth Water found (m): 64.62 Recommended Pumping Rate: 25 L/min

PART III: PUMP INSTALLATION DETAILS

Pump Type / HP (submersible, centrifugal, jet, etc): submersible

Date of Installation:

Pump Intake Depth (m):

Storage Tank Type (bladder, contact, etc.):

Tank capacity (specify units):

Have you had any problems with your pump? (If so, what?)

PART IV: WATER TREATMENT SYSTEM(S)

Do you have a water treatment system(s)? Yes / No (circle one)

Type(s): (pls circle) Water softener / Reverse Osmosis / Distillation / Filtration / UV

Date of Installation:

Services: Entire system / Kitchen Faucet (circle one or write other)

Water Softener: Salt Type: (pls circle) NaCl / KCl

Discharge location:

Reverse Osmosis:

Discharge location:

Filtration: (pls circle) Cartridge / Greensand / Other (specify):

Cartridge size (um):

How often is cartridge replaced?

Disinfection: Make/model:

Location:

PART V: WATER USAGE

What is well water used for (specify for each well)?

(eg., domestic supply, agricultural, commercial/industrial usage - see below; give specifics)

Domestic supply

If domestic usage, specify number of persons using well: 1-2

Lawn watering? ✓ (Y/N)

Type & number of Livestock watered from well:

Other uses for water not specified above: Bottling for personal use at permanent residence.

Owner Permission to Take Well Water Level

print name

signature

Well Water Level (m TOC):

Date/Time:

PART VI: SEPTIC SYSTEM

Location:

Age:

Any problem with system?

When was tank last pumped?

Interviewed By:

Matt Debees

Date:

Nov 3 2021

Ref. BluMetric Well Inspection Survey Form-V2.xls

Site Plan /Well Insp.- indicate well vs Septic location, site drainage, adjacent land use, N arrow, scale

Is surface drainage away from well? Yes / No

Well Casing Height (m)

Any Contaminant sources near well? (downspouts, animal waste, storage tanks, fill pipes, leaking equip.)

Well location photo taken? Yes / No

please see well Record

APPENDIX F

Laboratory Report of Analyses

DRAFT



C.O.C.: DW116170

REPORT No. B21-38554 (i)

Report To:

Blumetric Environmental
1682 Woodward Drive,
Ottawa ON K2C 3R8 Canada
Attention: Matt DeGeer

Caduceon Environmental Laboratories

285 Dalton Ave
Kingston Ontario K7K 6Z1
Tel: 613-544-2001
Fax: 613-544-2770

DATE RECEIVED: 23-Nov-21

JOB/PROJECT NO.: 220037

DATE REPORTED: 29-Nov-21

P.O. NUMBER:

SAMPLE MATRIX: Drinking Water

WATERWORKS NO.

			Client I.D.	2003-01			
			Sample I.D.	B21-38554-1			
			Date Collected	23-Nov-21			
Parameter	Units	R.L.	Reference Method	Date/Site Analyzed			
Fluoride	mg/L	0.1	SM4110C	26-Nov-21/O	0.2		
Chloride	mg/L	0.5	SM4110C	26-Nov-21/O	7.2		
Nitrite (N)	mg/L	0.1	SM4110C	26-Nov-21/O	< 0.1		
Nitrate (N)	mg/L	0.1	SM4110C	26-Nov-21/O	< 0.1		
Nitrate + Nitrite (N)	mg/L	0.1	SM4110C	26-Nov-21/O	< 0.1		
Sulphate	mg/L	1	SM4110C	26-Nov-21/O	18		
Hardness (as CaCO ₃)	mg/L	1	SM 3120	29-Nov-21/O	213		
Barium	mg/L	0.001	SM 3120	29-Nov-21/O	0.086		
Boron	mg/L	0.005	SM 3120	29-Nov-21/O	0.153		
Chromium	µg/L	2	SM 3120	29-Nov-21/O	< 2		
Copper	mg/L	0.002	SM 3120	29-Nov-21/O	0.019		
Zinc	mg/L	0.005	SM 3120	29-Nov-21/O	0.006		
Sodium	mg/L	0.2	SM 3120	29-Nov-21/O	28.7		
Calcium	mg/L	0.02	SM 3120	29-Nov-21/O	55.8		
Iron	mg/L	0.005	SM 3120	29-Nov-21/O	< 0.005		
Potassium	mg/L	0.1	SM 3120	29-Nov-21/O	2.8		
Magnesium	mg/L	0.02	SM 3120	29-Nov-21/O	18.0		
Manganese	mg/L	0.001	SM 3120	29-Nov-21/O	0.018		
Strontium	mg/L	0.001	SM 3120	29-Nov-21/O	0.557		
Fecal Coliform	cfu/100mL	1	SM9222D	24-Nov-21/K	0		
Dissolved Organic Carbon	mg/L		EPA 415.2	/			
Antimony	mg/L	0.0001	EPA 200.8	26-Nov-21/O	< 0.0001		
Arsenic	mg/L	0.0001	EPA 200.8	26-Nov-21/O	< 0.0001		
Beryllium	mg/L	0.0001	EPA 200.8	26-Nov-21/O	< 0.0001		
Cadmium	mg/L	0.000015	EPA 200.8	26-Nov-21/O	< 0.000015		
Cobalt	mg/L	0.0001	EPA 200.8	26-Nov-21/O	0.0001		
Lead	mg/L	0.00002	EPA 200.8	26-Nov-21/O	0.00018		

M. Dubien

R.L. = Reporting Limit

Test methods may be modified from specified reference method unless indicated by an *

Site Analyzed=K-Kingston, W-Windsor, O-Ottawa, R-Richmond Hill, B-Barrie

Michelle Dubien
Lab Manager

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C.O.C.: DW116170

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Blumetric Environmental
1682 Woodward Drive,
Ottawa ON K2C 3R8 Canada

Attention: Matt DeGeer

Caduceon Environmental Laboratories

285 Dalton Ave
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Tel: 613-544-2001
Fax: 613-544-2770

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SAMPLE MATRIX: Drinking Water

WATERWORKS NO.

			Client I.D.	2003-01			
			Sample I.D.	B21-38554-1			
			Date Collected	23-Nov-21			
Parameter	Units	R.L.	Reference Method	Date/Site Analyzed			
Molybdenum	mg/L	0.0001	EPA 200.8	26-Nov-21/O	0.0008		
Nickel	mg/L	0.0002	EPA 200.8	26-Nov-21/O	< 0.0002		
Selenium	mg/L	0.001	EPA 200.8	26-Nov-21/O	< 0.001		
Silver	mg/L	0.0001	EPA 200.8	26-Nov-21/O	< 0.0001		
Thallium	mg/L	0.00005	EPA 200.8	26-Nov-21/O	< 0.00005		
Uranium	mg/L	0.00005	EPA 200.8	26-Nov-21/O	0.00198		
Vanadium	mg/L	0.0001	EPA 200.8	26-Nov-21/O	0.0002		
Mercury	mg/L	0.00002	SM 3112 B	25-Nov-21/O	< 0.00002		
Total Coliform	cfu/100mL	1	MOE E3407	24-Nov-21/K	0		
E coli	cfu/100mL	1	MOE E3407	24-Nov-21/K	0		
Background	cfu/100mL	1	MOE E3407	24-Nov-21/K	0		
Conductivity @25°C	µmho/cm	1	SM 2510B	25-Nov-21/O	476		
Alkalinity(CaCO3) to pH4.5	mg/L	5	SM 2320B	25-Nov-21/O	234		
pH @25°C	pH Units		SM 4500H	25-Nov-21/O	8.12		
TDS (Calc. from Cond.)	mg/L	1	Calc.	26-Nov-21	246		
Ammonia (N)-Total	mg/L	0.01	SM4500-NH3-H	25-Nov-21/K	0.02		
Colour	TCU	2	SM 2120C	26-Nov-21/O	< 2		
Turbidity	NTU	0.1	SM 2130	25-Nov-21/O	0.2		

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Michelle Dubien
Lab Manager

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C.O.C.: DW116170

REPORT No. B21-38554 (ii)

Report To:

Blumetric Environmental
1682 Woodward Drive,
Ottawa ON K2C 3R8 Canada

Attention: Matt DeGeer

Caduceon Environmental Laboratories

285 Dalton Ave
Kingston Ontario K7K 6Z1
Tel: 613-544-2001
Fax: 613-544-2770

DATE RECEIVED: 23-Nov-21

JOB/PROJECT NO.: 220037

DATE REPORTED: 29-Nov-21

P.O. NUMBER:

SAMPLE MATRIX: Drinking Water

WATERWORKS NO.

			Client I.D.	2003-01			
			Sample I.D.	B21-38554-1			
			Date Collected	23-Nov-21			
Parameter	Units	R.L.	Reference Method	Date/Site Analyzed			
Acetone	µg/L	30	EPA 8260	26-Nov-21/R	< 30		
Benzene	µg/L	0.5	EPA 8260	26-Nov-21/R	< 0.5		
Bromodichloromethane	µg/L	2	EPA 8260	26-Nov-21/R	< 2		
Bromoform	µg/L	5	EPA 8260	26-Nov-21/R	< 5		
Bromomethane	µg/L	0.5	EPA 8260	26-Nov-21/R	< 0.5		
Carbon Tetrachloride	µg/L	0.2	EPA 8260	26-Nov-21/R	< 0.2		
Monochlorobenzene (Chlorobenzene)	µg/L	0.5	EPA 8260	26-Nov-21/R	< 0.5		
Chloroform	µg/L	1	EPA 8260	26-Nov-21/R	< 1		
Dibromochloromethane	µg/L	2	EPA 8260	26-Nov-21/R	< 2		
Dichlorobenzene, 1,2-	µg/L	0.5	EPA 8260	26-Nov-21/R	< 0.5		
Dichlorobenzene, 1,3-	µg/L	0.5	EPA 8260	26-Nov-21/R	< 0.5		
Dichlorobenzene, 1,4-	µg/L	0.5	EPA 8260	26-Nov-21/R	< 0.5		
Dichlorodifluoromethane	µg/L	2	EPA 8260	26-Nov-21/R	< 2		
Dichloroethane, 1,1-	µg/L	0.5	EPA 8260	26-Nov-21/R	< 0.5		
Dichloroethane, 1,2-	µg/L	0.5	EPA 8260	26-Nov-21/R	< 0.5		
Dichloroethylene, 1,1-	µg/L	0.5	EPA 8260	26-Nov-21/R	< 0.5		
Dichloroethene, cis-1,2-	µg/L	0.5	EPA 8260	26-Nov-21/R	< 0.5		
Dichloroethene, trans-1,2-	µg/L	0.5	EPA 8260	26-Nov-21/R	< 0.5		
Dichloropropane, 1,2-	µg/L	0.5	EPA 8260	26-Nov-21/R	< 0.5		
Dichloropropene, cis-1,3-	µg/L	0.5	EPA 8260	26-Nov-21/R	< 0.5		
Dichloropropene, trans-1,3-	µg/L	0.5	EPA 8260	26-Nov-21/R	< 0.5		
Dichloropropene 1,3-cis+trans	µg/L	0.5	EPA 8260	26-Nov-21/R	< 0.5		
Ethylbenzene	µg/L	0.5	EPA 8260	26-Nov-21/R	< 0.5		
Dibromoethane, 1,2- (Ethylene Dibromide)	µg/L	0.2	EPA 8260	26-Nov-21/R	< 0.2		
Hexane	µg/L	5	EPA 8260	26-Nov-21/R	< 5		

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Michelle Dubien
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SAMPLE MATRIX: Drinking Water

WATERWORKS NO.

			Client I.D.	2003-01			
			Sample I.D.	B21-38554-1			
			Date Collected	23-Nov-21			
Parameter	Units	R.L.	Reference Method	Date/Site Analyzed			
Methyl Ethyl Ketone	µg/L	20	EPA 8260	26-Nov-21/R	< 20		
Methyl Isobutyl Ketone	µg/L	20	EPA 8260	26-Nov-21/R	< 20		
Methyl-t-butyl Ether	µg/L	2	EPA 8260	26-Nov-21/R	< 2		
Dichloromethane (Methylene Chloride)	µg/L	5	EPA 8260	26-Nov-21/R	< 5		
Styrene	µg/L	0.5	EPA 8260	26-Nov-21/R	< 0.5		
Tetrachloroethane, 1,1,1,2-	µg/L	0.5	EPA 8260	26-Nov-21/R	< 0.5		
Tetrachloroethane, 1,1,2,2-	µg/L	0.5	EPA 8260	26-Nov-21/R	< 0.5		
Tetrachloroethylene	µg/L	0.5	EPA 8260	26-Nov-21/R	< 0.5		
Toluene	µg/L	0.5	EPA 8260	26-Nov-21/R	< 0.5		
Trichloroethane, 1,1,1-	µg/L	0.5	EPA 8260	26-Nov-21/R	< 0.5		
Trichloroethane, 1,1,2-	µg/L	0.5	EPA 8260	26-Nov-21/R	< 0.5		
Trichloroethylene	µg/L	0.5	EPA 8260	26-Nov-21/R	< 0.5		
Trichlorofluoromethane	µg/L	5	EPA 8260	26-Nov-21/R	< 5		
Vinyl Chloride	µg/L	0.2	EPA 8260	26-Nov-21/R	< 0.2		
Xylene, m,p-	µg/L	1.0	EPA 8260	26-Nov-21/R	< 1.0		
Xylene, o-	µg/L	0.5	EPA 8260	26-Nov-21/R	< 0.5		
Xylene, m,p,o-	µg/L	1.1	EPA 8260	26-Nov-21/R	< 1.1		

M. Dubien

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Michelle Dubien
Lab Manager

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DRAFT

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The Tower, The Woolen Mill
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Tel: 613.531.2725
Fax: 613.531.1852
kingston@blumetric.ca

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